

SEWER ONLY

# PROJECT MANUAL

## Technical Specifications

### CITY OF SPRING HILL, TENNESSEE

## STANDARD SPECIFICATIONS FOR SEWAGE ADDITIONS

MAYOR

HONORABLE RICK GRAHAM

BOARD OF ALDERMEN

BRUCE HULL, VICE MAYOR

JONATHAN DUDA

KEITH HUDSON

ELIOT MITCHELL

CHAD WHITTENBURG

KAYCE WILLIAMS

AMY WURTH

SUSAN ZEMEK

CITY ADMINISTRATOR

VICTOR LAY

DIRECTOR OF INFRASTRUCTURE

DAN ALLEN

TN DEPT OF ENVIRONMENT  
AND CONSERVATION

OCT 27 2014

DIV OF WATER RESOURCE  
RECEIVED



APPROVAL EXPIRES

NOV 19 2017

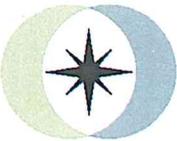
TN. DEPT. OF ENVIRONMENT & CONSERVATION  
DIVISION OF WATER POLLUTION CONTROL

APPROVAL OF CONSTRUCTION  
THIS STAMP HAS BEEN RECEIVED AND REVIEWED BY THE  
TENNESSEE DEPT. OF ENVIRONMENT & CONSERVATION  
DIVISION OF WATER POLLUTION CONTROL  
AND IS HEREBY APPROVED FOR CONSTRUCTION BY THE COMMISSIONER

NOV 19 2014

THIS APPROVAL SHALL NOT BE CONSTRUED AS CREATING A PRESUMPTION OF COR-  
RECT OPERATION OR AS WARRANTING BY THE COMMISSIONER THAT THE APPROVED  
FACILITIES WILL REACH THE DESIGNED GOALS FOR THE COMMISSIONER.

*Adrian Baber*



**Dempsey, Dilling & Associates, P.C.**

**Engineering Consultants**

502 HAZELWOOD DRIVE  
SMYRNA, TENNESSEE 37167  
TEL 615/220-5800 FAX 615/220-5888  
www.DEMPSEYDILLING.COM

WPC14-0800

CITY OF SPRING HILL, TENNESSEE

Approved By: *[Signature]*

Title: City Administrator

Date: 10/23/14

Date: October, 2014

File No.: 0100-252

## INDEX TO PROJECT MANUAL

These specifications give the minimum requirements for installation of water and sewer additions in the Town of Spring Hill, Tennessee.

Any special construction problems or conditions not covered under these specifications shall be submitted in writing to the Town of Spring Hill for approval.

The Standard Drawings are part of these specifications and all construction shall conform to the details shown on these drawings.

### General Specifications

| <b>DESCRIPTION</b>  | <b>PAGES</b> |
|---|--------------|
| <b>DIVISION 1: GENERAL REQUIREMENTS</b>   |              |
| 01010 General Guidelines Covering Installation of Sewer Lines and Appurtenances | 19           |
| 01031 Special Project Procedures  | 4            |
| 01090 Reference Standards   | 5            |
| 01400 Quality Control   | 3            |
| 01568 Erosion Control   | 3            |
| 01620 Storage and Protection  | 1            |

### Technical Specifications

|   |    |
|---|----|
| <b>DIVISION 2: SITE WORK</b>                |    |
| 02221 Unclassified Excavation for Utilities | 10 |
| 02485 Seeding                               | 3  |
| 02575 Pavement Repair                       | 3  |
| 02600 Manholes                              | 5  |
| 02722 Sanitary Sewers                       | 13 |
| 02724 Sewage Force Main                     | 9  |
| 02725 Boring and Casing for Sanitary Sewers | 3  |
| <b>DIVISION 3: SITE WORK</b>                |    |
| 03303 Concrete for Utility Lines            | 1  |

**STANDARD DRAWINGS**

- F-1 Typical Fence and Gate Detail (3 Sheets)
- FM-1 Force Main Air Release/Air Vacuum Valve Detail
- GS-1 Standard Precast Concrete Manholes (2 Sheets)
- GS-2 Plastic Gasket Joint
- GS-2.1 Typical Manhole Step Detail
- GS-2.2 Manhole Step Detail
- GS-2.4 Drop Assembly for Standard Manholes
- GS-2.5 Standard Manhole Vent
- GS-3 Frame and Cover
- GS-4 Sanitary Sewer Service Line Connection Details
- GS-5.4 Typical Concrete Waterstop for Gravity Sewer Lines
- GS-8 Pipe to Manhole Connection Detail
- PR-1A Pavement Replacement Detail (Bituminous Base with Surface)
- PS-4 Standard Connection of Force Main to Manhole

**S  
P  
E  
C  
I  
F  
I  
C  
A  
T  
I  
O  
N  
S**

CITY OF SPRING HILL  
MAURY COUNTY, TENNESSEE

GENERAL GUIDELINES  
COVERING THE INSTALLATION OF UTILITY LINES  
AND APPURTENANCES

(Revised October 2014)

A. GENERAL GUIDELINES

The purpose of these guidelines is to provide a guide to the Developers and their engineers and contractors in order to achieve an acceptable installation for furnishing of utility service to subdivisions and other developments. The words "A/E," "Owner," "City of Spring Hill," and "Superintendent of Water and Sewer Systems" are to be used interchangeably. Summarized below are requirements and conditions that apply to the granting of utility service by the City of Spring Hill.

1.1 Prior to the design of any utility line extension or expansion, the design engineer should first confer with the City of Spring Hill Planning Commission in regard to growth potential and density that may be expected in the general area of the extension being planned. A conference with the Superintendent of Water and Sewer Systems should follow to discuss system standards and requirements, as well as any problems related to the mains being extended.

1.1.1 Construction of utility lines, including individual service connections, may not begin prior to approval by the City of Spring Hill.

1.2 No connection to an existing utility shall be made until all lines have been completely tested and the tie-in is approved by the Project Inspector.

1.3 The City of Spring Hill will not accept utility lines that were not approved in accordance with the City Code and constructed in accordance with these specifications.

1.4 The City of Spring Hill requires the following bonds (or certified cashier's check):

1.4.1 Performance Bond - Contractor/Developer will be required to provide a Maintenance Bond for one (1) year after inspection and acceptance by the City of the improvements in the amount of 25% of construction cost.

0100-252  
01010-1

1.4.2 Road Repair Bond - 100% of estimated roadway (public) repair cost, amount to be set by codes department, to be returned to Contractor/Developer upon acceptance of the repaired/replaced roadway by the Public Works Department.

1.4.3 Warranty Bond - The Warranty Bond shall be 25% of the Performance Bond; to be returned to Contractor/Developer at end of warranty period. The typical warranty period is for one year from the date of substantial completion.

1.5 Service connection and service line construction to property line or right-of-way (only) is covered herein. Service line constructed from property line or right-of-way to structure is covered in the latest edition of the Standard Plumbing Code.

1.6 Under the terms of the Spring Hill Municipal Code, water service may be denied to structures connected to a sewer line or service not accepted by the City.

1.7 All utility lines and services (to property line or right-of-way only) constructed utilizing these specifications become the property of the City of Spring Hill upon acceptance by the City. Utility lines and services (to property line or right-of-way only) will not be accepted by the City unless and until they are in strict conformance with these specifications.

1.8 Three (3) sets of plans and specifications, including a vicinity map, shall be submitted for the initial review. If the plans are in order, with no major changes, the Developer or his Engineer will submit the number of additional sets of plans needed for the project for approval.

1.9 Five (5) sets of drawings including vicinity map shall be submitted for approval. Submittals shall be at least fourteen (14) days prior to a scheduled meeting in order to be considered at that meeting. Contractor's developers, and others are asked to submit drawings as far in advance as possible in order to conserve time at planning and commission meetings. After approval, four (4) sets of drawings shall be submitted to the Tennessee Department of Environment and Conservation for their approval. Approval of the plans and specifications by the Tennessee Department of Environment and Conservation, Tennessee Department of Transportation, Railroads, Corps of Engineers, Tennessee Valley Authority, and any other agency having jurisdiction is required before beginning construction. One (1) state approved set of drawings and one (1) copy of the State approval letter shall be provided to the Superintendent of Water and Sewer Collections Systems prior to beginning construction. Prior to acceptance of lines by the City,

0100-252  
01010-2

one set of reproducible "Record Drawings" showing all work, changes, service locations, and other data not shown on the original set shall be given to the Superintendent of Water and Sewer Collections Systems after each project or phase of a project is completed.

1.10 Detail drawings and specifications shall be submitted by the A/E employed by the Developer for any special condition or structures such as pump stations, creek crossings, etc., and approved by the City before beginning any construction.

1.11 Easements required across private property or in roads are to be acquired by the Developer in the name of the City. Easements shall have a minimum width of 20 feet. Wider easements may be required for sewer lines over 12 feet deep.

1.12 All applicable Federal and State laws, municipal ordinances, and the rules and regulations of all authorities having jurisdiction over construction of the project shall apply to the construction throughout.

1.13 Sizes and locations of all water and sewer lines and appurtenances, and all construction shall be in accordance with the plans approved by the City.

1.14 Permits for pavement cuts or crossing of public roads, including any special backfill and pavement repair as required by the agency having jurisdiction, are the responsibility of the Developer. A bond is required from the Developer to cover all costs of repair and maintenance for a period of one (1) year from the date of acceptance of the project for all work performed in existing rights-of-way of all roads.

1.15 If construction has not started within one (1) year from the date of approval, utility plans shall be resubmitted to renew approval. Renewal is not guaranteed.

1.16 The Contractor's name, project cost, and estimated working time for each project shall be submitted to the City.

1.17 Laboratory test reports shall be provided on all pipe to assure that it meets the requirements of the City's specifications.

1.18 Shop drawings for utility materials shall be submitted to the City of Spring Hill for review after being thoroughly checked by the Contractor and stamped with his approval.

0100-252  
01010-3

1.19 The City reserves the right to relocate water and sewer lines on the construction plans to facilitate maintenance.

1.20 All utility construction shall be in accordance with specifications of the City of Spring Hill.

1.21 All grading work shall be completed and all roads constructed to subgrade and lot corners are to be marked prior to the installation of utility lines.

1.22 The contractor shall be responsible for locating and verifying the elevations of existing utilities prior to construction.

1.23 The Developer's Engineer shall provide a complete set of Record Drawings; one compatible electronic digital copy; one set of reproducible and two sets of blue line/black line drawings, upon completion of construction and they shall include actual field angles between lines, all actual service lines and tee locations, the distance of the end of service lines to property corners and lines, the depth to top of the end of the service line, and shall reflect all alignment and grade changes. This item must be completed and submitted prior to acceptance of the sewers or water mains into the public system and any connections being made thereto.

1.24 The Contractor shall provide a set of construction cut sheets prior to the preconstruction meeting and the cut sheets shall include the stations of all proposed service connections.

1.25 A one (1) year warranty period will begin upon the date of acceptance of the project by the City.

1.26 Any special requirements shall be transmitted as a part of the approval.

1.27 All plans shall be stamped by a Tennessee Licensed Professional Engineer.

2. Initial Plan Submittals: The plans must be submitted at least twenty-one (21) days prior to the date on which action is desired. The initial submittal should include, but not be limited to the following:

2.1 Three (3) copies of the plan.

2.2 Specifications.

0100-252  
01010-4

2.3 Engineering reports including design criteria used in sizing mains, and/or pumping stations.

### 3. Easements

3.1 When utility lines are constructed outside a public right-of-way, easements must be a minimum of 20 feet in width.

3.2 Easements for utility line extensions may be provided in either of two (2) ways.

3.2.1 Easement Document on form, approved by the City, which must include legal description of the easement(s), legal owner's name and Book and Page where deed is recorded, and must be signed by the Owner, and then notarized.

3.2.2 Record with Subdivision Plat - If this method of recording easements is chosen, a preliminary plat of the subdivision must be provided at the time of plans submittal, which clearly defines the easements to be recorded, along with a letter of intent from the Licensed Engineer or Licensed Surveyor who will stamp the final subdivision plat, assuring that easements will be recorded as shown on the preliminary plat.

3.3 All easements must be obtained and recorded in developed areas before construction can begin. In new subdivisions the letter of intent and preliminary plat showing the easements will be sufficient to start construction. However, the Final Plat must be recorded prior to final acceptance of the new facilities.

3.4 Special easements such as Railroad Crossings, T.V.A. crossings and State Highway crossings must be prepared by the Developer's Engineer.

### 4. Pre-Construction Meeting

4.1 Before beginning any construction, the Developer shall contact the City and execute a contract with them paying all tapping privilege fees as required. After this contract is executed and before beginning any construction, the Developer or his Engineer shall schedule a pre-construction conference to be held between the Contractor, Developer, Developer's Engineer, and the City and their Engineer. At this meeting, the Contractor will be informed of the City's policies and any special requirements. Listed below is a CHECKLIST of items relating to the project:

0100-252  
01010-5

4.2 BEFORE Pre-Construction Conference:

- 4.2.1 Developer is to coordinate conference.
- 4.2.2 Developer, or his Engineer, is to have project plans approved by all agencies.
- 4.2.3 Developer is to have a contract with the utility contractor prior to the preconstruction meeting.
- 4.2.4 Contractor is to have shop drawings approved by the City.
- 4.2.5 When submitting plans and shop drawings to the City's Engineers, they will retain one (1) copy and the City will retain two (2) copies. Shop drawings will not be reviewed unless they have been checked by the Contractor and stamped by him to indicate that they meet the specifications.
- 4.2.6 Developer is to have at conference:
- 4.2.7 Approved plans.
- 4.2.8 Copy of Contractor's contract (both off-site and on-site).
- 4.2.9 Tap fees and Impact fees. All fees are subject to final approval by the City of Spring Hill Board of Aldermen.

4.3 To Attend Conference:

- 4.3.1 Developer.
- 4.3.2 Developer's Engineer.
- 4.3.3 Developer's Contractor.
- 4.3.4 Representative from the City's Engineer.
- 4.3.5 Representative of the City of Spring Hill and the Project Inspector.

B. SANITARY SEWER GENERAL SPECIFICATIONS

1. Sewer Extension and/or Service Connection: The following are guidelines for the preparation of sanitary sewer plans and should

not be construed as being the total requirements. The City may at its option require additions to be made in the plans where circumstances warrant.

2. Plans shall be drawn on a standard 24" x 36" sheet.

3. A cover sheet shall be made a part of all plans, and shall incorporate a location map on an approximate scale not less than 1" = 1,000', the name of the project and, the names, addresses and telephone numbers of the Developer and the Engineer.

4. Include a key map indicating sheet numbers for each sewer line.

5. Sewer plans must be on plan and profile sheets, with contour lines shown in the plan portion and the lowest elevation of the sewer line beginning on the left side of the sheet in the profile.

6. All plans must show the locations of the existing and proposed utilities including, but not limited to, gas lines, underground telephone conduits, power and telephone poles, water mains, sanitary sewer lines, storm sewers, etc.

7. The scale of the plan/profile sheet will be: Plan 1" = 50' horizontal, Profile 1" = 5' or 1" = 10' vertical.

8. All sewer plans shall include at least one (1) bench mark based on U.S.G.S. Datum. Additional bench marks shall be shown at approximately 1,500 feet intervals. The use of a manhole invert elevation or an assumed elevation will not be approved.

9. Show all topographic features, such as driveways, pavement, rights-of-way, property lines, storm drainage structures, etc.

10. The direction of North should be clearly shown on all plans.

11. All property lines should be shown on the plans and each parcel should show the map and parcel number, lot number and/or house number.

12. A connection must be provided for each parcel or proposed lot. The connection will be shown as a SDR 26 PVC tee wye (machine made only) and a four (4) inch residential or six (6) inch commercial SDR 26 PVC service line extension therefrom where applicable. Hand made tees and "Y" connections are not acceptable. When sewers are constructed by private developers to serve proposed developments and are to be construed as public mains within the public right-of-way, the Developer will provide a 4" by 8" wye with schedule 40 PVC

0100-252  
01010-7

to serve all parcels of property which lie along said main extension (which can be provided gravity service by said main). When laying the mains in private property, a wye and ten (10) feet of 4-inch service line shall be provided for each existing parcel. Commercial properties require 6" by 8" wyes and service lines.

13. A maximum of only two (2) service lines will be allowed into permanent end manholes, and a minimum 45 degree alignment differential must be maintained between them. At no time will an angle less than 90 degrees be permitted between them and the out or downstream sewer main. The service lines must enter the manhole within 1.9 feet of the base of the manhole and the invert must be properly shaped for them. The maximum length of a service line from the sewer main to property line shall be seventy-five (75) feet.

14. Special pipe considerations are as follows where Class 250 Ductile iron Pipe will be installed in place of SDR 26 PVC Pipe:

14.1 In areas which have been filled and the proposed pipe will be within the fill, Class 250, ductile iron must be specified.

14.2 If ductile iron pipe is specified for any part of a sewer, then it must be specified from manhole to manhole; jointing of two different type pipes between manholes will not be permitted.

14.3 Due to maintenance considerations, it will be City's policy to require that all lateral sewers proposed at depths greater than 20 feet be constructed of Class 250 ductile iron pipe and any service line risers from this depth also be ductile iron pipe. This condition should be avoided whenever possible and first consideration given to other routes.

14.4 All sanitary sewers shall have a minimum of 30 inches cover in private property and 48 inches in paved areas subject to vehicular traffic. Across drains and areas where cover is less than 30 inches, ductile iron pipe or concrete encasement will be required.

15. Manholes shall be installed at the upper end of each line, at all changes in grades, size or alignment, at all intersections, and at distances not greater than 350 feet for sewer 15 inches in diameter or less, 400 feet for sewers 18 inches to 21 inches in diameter, and 500 feet for sewers over 24 inches in diameter.

16. When sewers are proposed along drains and lie within a potential flood plain or lie adjacent to a drainage ditch or drainage structure in which there is a potential problem of storm water entering the sanitary sewer, the City will require approved watertight frames and covers be installed on the manholes.

17. A vent stack assembly will be required on water tight manholes at 1,000 feet intervals.

18. When sewers are proposed to serve new subdivisions, contour elevations must be shown on the sewer plans. At least one (1) copy of the subdivision grading and drainage plan and a copy of the road plans must be submitted with the sewer plan for review, and must contain a typical section of the proposed roadway. A statement should be incorporated into the letter for transmittal for plans designating which roads are to be public and which are to be private, as well as designating which sewer lines are to be public.

19. Smaller lines shall not be connected to larger lines by utilizing a concrete collar. Only an approved compression or rubber O-ring style coupling will be acceptable. The practice of "hammer tapping" a sewer line is not in conformance with the Standard Plumbing Code and is not an acceptable method of connecting a service line to a new or existing sewer line. In all cases, a tee, wye, or tapping saddle shall be used. Contractors and/or plumbers caught or suspected of utilizing either illegal practice hereinbefore discussed will be asked to provide a guarantee bond as specified in 1.4.3 hereinbefore prior to being allowed to complete improvements to the Spring Hill Sewer System.

20. Any time sewer lines are proposed to serve property where the "serviceability" of a lot or residence is questionable, the lot or residence must be identified with the following note: The service tee is to be placed at the lowest possible elevation on the main line and the service line is to be laid on a minimum slope. The home builder is responsible for locating the elevation of the end of the service line and setting building finished floor elevations such that gravity service is available. This note is also to be put on the recorded plat identifying critical lots.

21. The profiles of all drains adjacent to and crossing proposed sewers must be shown on the sewer plan profile. Concrete protection must be provided on sanitary sewers across drains where there will be less than 2.5 feet of cover.

22. Whenever wastewater lift stations are proposed, the following information must be submitted.

0100-252  
01010-9

22.1 Complete design criteria for the proposed pumping station including, but not limited to, the following:

22.1.1 Topographic map with the drainage area clearly defined and the acreage shown.

22.1.2 Complete information concerning the proposed area of service, including the number and type of proposed units.

22.1.3 Complete anticipated flow data based on Tennessee Department of Environment and Conservation design criteria and utilizing a peak factor 2.5.

22.1.4 Complete details of possible alternate gravity sewers to serve the same area, including cost estimates of both type systems.

23. In general, wastewater collection extensions shall be designed for the estimated ultimate tributary population.

C. DESIGN CRITERIA

1. Design Factors: In determining the required capacities of sanitary sewers, the following factors must be considered:

1. Maximum hourly quantity of wastewater.
2. Additional maximum wastewater from industrial plants.
3. Ground water infiltration.

2. Design Basis

Per capita flow: Sewer systems serving residential development should be designed on the basis of an average daily per capita flow of wastewater of not less than 100 gallons per day when no water use information is available. This amount of flow is assumed to cover nominal infiltration, but an additional allowance should be made where conditions are unfavorable.

Generally, the sewers should be designed to carry, when running full, not less than the following daily per capita contributions of wastewater, exclusive of wastewater from industrial plants:

0100-252  
01010-10

1. Laterals and sub-main sewers; 400% of average design flow.

2. Main, trunk & outfall sewers: 250% of average design flow.

3. Minimum Size

No sewer collection line shall be less than eight (8) inches in diameter.

4. Depth

In general, sewers should be deep enough to drain basements and to prevent freezing. Where practical, a minimum depth of five (5) feet should be maintained.

5. Slope

All sewers shall be designed and constructed to give mean velocities, when flowing half full, of not less than 2.0 feet per second. The minimum required slopes for 8 inch through 12 inch sewer mains are shown below. However, these slopes should be used only when required. All sewers shall be laid with uniform slope between manholes.

| <u>Sewer Size</u><br>(inches) | <u>Minimum Slope</u><br>(feet per 100 feet) |
|-------------------------------|---|
| 8                             | 0.40  |
| 10                            | 0.28  |
| 12                            | 0.22  |
| 15                            | 0.15  |
| 18                            | 0.12  |
| 21                            | 0.10  |
| 24                            | 0.08  |
| 27                            | 0.067                                       |
| 30                            | 0.058                                       |
| 36                            | 0.05  |
| 42                            | 0.042                                       |

6. Alignment

Sewers shall be designed with straight alignment between manholes.

7. Increased Size

When a smaller sewer joins a larger one, the invert of

0100-252  
01010-11

the larger sewer should be lowered sufficiently to maintain the same energy gradient. An acceptable approximate method for securing these results is to place the 0.8 depth point of both sewers at the same elevation.

8. High Velocity Protection

Ductile iron pipe shall be used when slopes are greater than:

| <u>SEWER SIZE INCHES</u> | <u>SLOPE (FT/100 FT)</u> |
|--------------------------|--------------------------|
| 8                        | 18                       |
| 10                       | 13                       |
| 12                       | 9                        |

9. Pipe Bedding

All sewers shall be designed to prevent damage from superimposed loads. Proper allowance for loads on the sewer shall be made because of the width and depth of trench. Backfill material from one (1) foot above the pipe should not exceed six (6) inches in diameter at its greatest dimension. As a general rule, in roadways where cover is less than four (4) feet, or in open areas where cover is less than 2 1/2 feet, ductile iron pipe or concrete encasement shall be used. Ductile iron pipe shall be required when sewer installation occurs in areas of non-virgin soil (i.e. areas of "fill"). Piers shall be provided for when necessary for support. An impermeable barrier of compacted clay or concrete encasement shall be used at the transition from fill to virgin soil to prevent piping of water through the crushed stone bedding.

For structural reasons, ductile iron pipe, concrete encasement, or relocation shall be required when culverts or other conduits are laid such that the top of the sewer is less than 18 inches below the bottom of the culvert or conduit. Special care shall be used in placing bedding in the haunching region.

1. Ductile Iron Pipe: Each sewer pipe section shall be laid on six (6) inch bed of size no. 7 or size no. 67 crushed stone and shall be backfilled to the springline of the pipe using size no. 7 or size no. 67 compacted crushed stone.
2. PVC Pipe: Each sewer pipe section shall be completely encapsulated with six (6) inches of

0100-252  
01010-12

bedding material on the top, both sides, and the bottom of the pipe. Bedding materials shall be size no. 7 or size no. 67 crushed stone.

3. Backfill material above the pipe envelopes shall consist either of fine, loose earth like sandy soil or loam or of granular material that is free from clods, vegetable matter, debris, stone, and/or objectionable materials and that has a size of no more than 6-inches. Place this backfill simultaneously on either side of the trench in even layers that before compaction are no more than 8 inches deep. Thoroughly and completely tamp each layer into place before placing additional layers.
4. When shown on the drawings, this backfill shall, at locations beneath or closely adjacent to pavement, consist of No. 67 (TDOT) crushed stone. Compaction of backfill material layers shall be at 98% by standard proctor test. Where adjacent to and within paved areas the top 12-inches of the trench at subgrade shall consist of crusher-run stone compacted at 98% by standard proctor test. Compaction testing shall be at intervals not greater than 500 feet along the trench and/or at spacing as directed by the site inspector.
5. From 1 foot above the pipe upward, the backfill material may contain broken stones that make up approximately 3/4 of the backfill total volume. However, if this type of backfill is used, there must be enough spalls and earth materials to fill all voids completely. The maximum dimension of individual stones in such backfill shall not exceed 6 inches, and the backfill material shall be placed and spread in even layers not more than 12 inches deep. At locations beneath or closely adjacent to pavement or at locations of improvements subject to damage by displacement, tamp and thoroughly compact the backfill in layers that, before compaction, are 6 inches deep. In other areas, the backfill for the upper portion of the trenches may be placed without tamping but shall be compacted to a density equivalent to that of adjacent earth material as determined by laboratory tests. Use special care to prevent the operation of backfilling equipment from causing any damage to the pipe.

0100-252  
01010-13

10. Joints and Infiltration

Sewer joints should be designed to minimize infiltration and to prevent the entrance of roots. Standard laying lengths for PVC pipe shall be not exceed 13.5 feet.

11. Air Pressure Testing

Low pressure air exfiltration testing of all pipes shall be as specified in ASTM C828-80. The pressure drop shall be calculated as the number of seconds for the air pressure to drop from a stabilized pressure of 3 1/2 psig to 2 1/2 psig.

MINIMUM TEST TIME FOR VARIOUS PIPE SIZES  
(Based upon ASTM C828-80)

| <u>Nominal Pipe Size (Inches)</u> | <u>Time (Min./100 feet)</u> |
|-----------------------------------|-----------------------------|
| 6                                 | 0.7                         |
| 8                                 | 1.2                         |
| 10                                | 1.5                         |
| 12                                | 1.8                         |
| 18                                | 2.0                         |
| 24                                | 3.0                         |

12. Manholes

- (a) Location: Manholes shall be installed at the upper end of each collection sewer line, at all changes in grade, at points of changes in size, and at all pipe intersections.
- (b) Drop Manholes: A drop pipe shall be provided for a sewer entering a manhole at an elevation of 24 inches or more above the manhole invert. Where the difference in elevation between the incoming sewer and the manhole invert is less than 24 inches, the invert should be u-shaped to prevent deposition of solids.
- (c) Diameter: The minimum diameter of manholes shall be 48 inches. The entrance tube shall be at least 24 inches in diameter.

13. Protection of Water Supplies

- (a) Water Supply Interconnections: There shall be no physical connection between a potable water supply line and a sewer or appurtenance thereto which

would permit the passage of any wastewater or polluted water into the potable supply.

(b) Relation to Water Mains:

1. Horizontal Separation: Whenever possible, sewers should be laid at least ten (10) feet horizontally from any existing or proposed water pipe. Should local conditions prevent a lateral separation of ten (10) feet to the water main if it is laid in a separate trench and if the elevation of the top of the sewer pipe is at least 18 inches below the bottom of the water pipe.
2. Vertical Separation: Whenever a sewer must cross under a water main, the sewer shall be laid at such elevation that the top of the sewer is at least 18 inches below the bottom of the water main. When the elevation of the sewer cannot be varied to meet the above requirement, the water main shall be relocated to provide the separation or reconstructed with ductile iron pipe for a minimum distance of ten (10) feet on each side of the sewer. At least one (1) full length of water main should be centered over the sewer so that both joints shall be as far from the sewer as possible.
3. When it is impossible to obtain proper horizontal and vertical separation as stipulated above, both the water main and the sewer shall be constructed of ductile iron pipe and shall be pressure-tested to assure watertightness.

14. Force Mains

- (a) Velocity: At design flow, velocity in excess of two (2) feet per second shall be maintained.
- (b) Air Release Valve: An automatic air release valve shall be placed at high points in the force main to prevent air-locking.
- (c) Termination: Force mains shall terminate in the invert of a manhole.

0100-252  
01010-15

- (d) Pipe Diameter: Force mains are to be a minimum of four (4) inches in diameter.
- (e) A maximum Hazen and Williams "C" factor used should not be greater than 130 regardless of that actually determined for the pipe.
- (f) Force mains using minimum four (4) inch ductile iron, cement-mortar lined, Class 50, slip-on type joint meeting the latest requirements of AWWA Standard C151 with a minimum of three (3) feet of cover will be acceptable to the City of Spring Hill.
- (g) For detection purposes, a 14 gage solid strand copper tracing wire (shielded) and an approved metallic tape shall be identified as "sewer" and be installed as per the manufacturer's instructions. Bury tape 12 inches below subgrade. Connections between wires shall be soldered or connected with wire nut fasteners and wrapped.

15. Wastewater Lift Stations

Wastewater lift station design criteria is not provided under these Standards. However, lift stations shall be of the wet well/dry sump configuration. Construction of the lift station shall include a paved (asphalt or concrete) driveway, minimum eight (8) feet high chain-link fence enclosing the site, minimum 12 feet wide gate for access, and a permanent potable water supply. The City will evaluate separately the materials and criteria proposed for use in the design of wastewater lift stations. Plans and specifications must be submitted to the City for approval. Once approval has been given by the City, plans and specifications must be submitted to the Tennessee Department of Environment and Conservation, Division of Water Pollution Control, for approval.

16. Means of Detecting PVC pipe

When PVC pipe is installed a minimum size 14 gauge copper wire shall be installed along the pipe. The ends of the wire shall terminate in a valve box or other acceptable location whereby detection equipment may be attached.

0100-252  
01010-16

## 17. Separation of Water Mains and Sewers

### (a) General:

The following factors should be considered in providing adequate separation:

1. Materials and type of joints for water and sewer pipes.
2. Soil conditions.
3. Service and branch connections into the water main and sewer line.
4. Compensating variations in the horizontal and vertical separations.
5. Space for repair and alterations of water and sewer pipes.
6. Off-setting of pipes around manholes.
7. Water mains and sanitary or storm sewers shall not be laid in the same trench.
8. Water and sewer services shall maintain the same separation as mains.

### (b) Parallel Installation:

1. Normal conditions-Water mains shall be laid at least ten (10) feet horizontally from any sanitary sewer, storm sewer or sewer manhole. Whenever possible; the distance shall be measured edge-to-edge.
2. Unusual conditions-When local conditions prevent a horizontal separation of ten (10) feet, a water main may be laid closer to a storm or sanitary sewer provided that:
  - i. The bottom of the water main is at least 18 inches above the top of the sewer.
  - ii. Where this vertical separation cannot be obtained, the sewer shall be constructed of materials and with joints that are

0100-252  
01010-17

equivalent to water main standards of construction and shall be pressure tested to assure watertightness prior to backfilling.

(c) Crossing:

1. Normal conditions-Water mains crossing house sewers, storm sewers, or sanitary sewers will be laid to provide a separation of at least 18 inches between the bottom of the water main and the top of the sewer, whenever possible.
2. Unusual conditions-When local conditions prevent a vertical separation as described hereinbefore, the following shall be used:
  - i. Sewers passing over or under water mains should be constructed of ductile iron.
  - ii. Water mains passing under sewers shall, in addition, be protected by providing a vertical separation of at least 18 inches between the bottom of the sewer and the top of the water main; adequate structural support for the sewers to prevent excessive deflection of joints and settling on the breaking the water mains; that the length of water pipe be centered at the point of crossing so that the joints will be equidistant as far as possible from the sewer. Both the sewer and the water main shall be constructed of water pipe and tested in accordance with these Standards.

(d) Sewer Manholes:

No water pipe shall pass through or come into contact with any part of sewer line or sewer manhole.

18. Surface Water Crossings

Surface water crossings, both under and over water, present special problems which should be discussed with the City of Spring Hill; the Tennessee Department of Environment and Conservation, Division of Water Supply and Division of Water Pollution Control; and the U.S.

0100-252  
01010-18

Army Corps of Engineers before plans are prepared.

All surface water crossings shall be in accordance with the requirements of the General Permit for an Aquatic Resource Alteration Permit.

(a) Above Water Crossings-The pipe shall be:

1. Adequately supported.
2. Protected from damage and freezing.
3. Accessible for repairs and replacement.

(b) When Crossing Water Courses which are greater than 15 Feet in Width:

1. The pipe shall be of special construction, having flexible, watertight joints;

END OF SECTION

0100-252  
01010-19

SECTION 01031

SPECIAL PROJECT PROCEDURES

1. SMOKING AND FIRE PRECAUTIONS

1.1 No smoking, fire or use of any fire- or explosion-producing tools or equipment will be permitted on the properties of oil companies or other concerns prohibiting same on their premises or at any locations where such may endanger said premises or the current operations thereon.

2. MANUFACTURERS' QUALIFICATIONS

2.1 The manufacturers of all materials and equipment used must be reputable and regularly engaged in the manufacture of the particular material or equipment for the use and service to which it will be subjected.

3. DEVELOPER SHALL PAY FOR ALL LABORATORY INSPECTION SERVICE

3.1 All materials and equipment used in the construction of the project shall be subject to adequate inspection and testing in accordance with accepted standards. The laboratory or inspection agency shall be selected by the Developer and approved by the Owner and A/E. The Developer shall pay for all laboratory inspection services as a part of the Contract. Submit all material test reports to the A/E in triplicate.

4. COMPLIANCE WITH STATE AND LOCAL LAWS

4.1 Comply with all applicable requirements of state and local laws and ordinances to the extent that such requirements do not conflict with federal laws or regulations.

5. MARKERS

5.1 Preserve all Corps of Engineers, USGS, TVA, State of Tennessee, and private markers; do not remove or disturb any such markers without prior approval from the A/E. Any removal and replacement of such markers shall be at the expense of the Developer.

6. PAVEMENT REPAIR AND/OR REPLACEMENT

6.1 Open cut pavement is not allowed and roadway bores nare required for roadway crossings.

6.1 If the City of Spring Hill allows an open cut due to special approved circumstances, pipe trenches shall be cut across or along existing pavement or shoulders, backfill same and restore traffic over the cuts as quickly as possible by constructing a temporary twelve-inch (12") surface of Class A, Grade D crushed stone. Add material and otherwise maintain such surface until the permanent pavement is restored or until the entire project is accepted. Temporary pavement may be required if open cut trench is not properly maintained until permanent pavement can be installed.

7. APPROVED CHEMICALS

7.1 All chemicals used during project construction or furnished for project operation, whether herbicide, pesticide, disinfectant, polymer, reactant, or of other classification, must show approval of either EPA or USDA. The use of all such chemicals and the disposal of residues shall be in strict conformance with all applicable instructions and regulations.

8. DEPARTMENT OF TRANSPORTATION PERMITS

8.1 The Owner will assist in securing any permits and provide bond as required by the Tennessee Department of Transportation for the installation of permanent facilities on State highway rights-of-way. The costs for such bonds and/or permits, if applicable, shall be paid by the Developer. All such work shall be coordinated with and be subject to the approval of the Tennessee Department of Transportation, in addition to the approval of the A/E.

8.2 The Developer will secure any permits as required by the local highway department for the installation of water lines within the rights-of-way of county roads. The Developer shall be responsible for complying with the requirements of the local highway department, and all such work shall be coordinated with and be subject to the approval of the local highway department, in addition to the approval of the Owner.

9. INSTALLATION, TESTING, AND GUARANTEE

9.1 The completely installed system shall be guaranteed against any and all defects of manufacture, materials, workmanship, or installation for a period of one year from the date of acceptance.

10. DRAWINGS OF RECORD

10.1 The Developer shall provide and keep up-to-date a complete record set of blue-line prints, which shall be corrected daily to

show every change, and the approved shop drawings. Keep this set of prints at the job site, and use only as a record set. This shall not be construed as authorization for the Developer to make changes in the approved layout without definite instructions in each case. Turn the set over to the Owner upon completion of the project.

#### 11. DETECTION WIRE

11.1 For detection purposes, a 14 gage solid strand copper tracing wire (shielded) shall be installed as per the manufacturer's instructions. Connections between wires shall be soldered or connected with wire nut fasteners and wrapped. Also, metallic tape marked "sewer" shall be provided 12" below grade directly above the force main shall be provided.

#### 12. UTILITIES

12.1 The Developer shall contact the owner of all underground utilities before beginning construction in the area. Carefully protect from damage all utilities in the vicinity or the work at all times. If it is necessary to repair, remove, and/or replace any such utility in order to complete the work properly, do so in compliance with the rules and regulations of the particular utility involved. Any such work shall be considered incidental to the construction of the project, and no additional payment will be allowed therefor.

#### 13. INSURANCE

The Contractor shall procure, maintain, and furnish an Owner's protective policy as hereinafter specified:

Owner's General Public Liability and Property Damage Insurance including vehicle coverage issued to the Owner and protecting the Owner from all claims for personal injury, including death, and all claims for destruction of or damage to property, arising out of or in connection with any operations under the Contract Documents, whether such operations be by the Contractor or by any Subcontractor employed by the Contractor or anyone directly or indirectly employed by the Contractor or by a Subcontractor employed by the Contractor. Insurance shall be written with a limit of liability of not less than \$1,000,000 for all damages arising out of bodily injury, including death, at any time resulting therefrom, sustained by any one person in any one accident; and a limit of liability of not less than \$1,000,000 aggregate for any such damages sustained by two or more persons in any one accident. Insurance shall be written with a limit of

liability of not less than \$500,000 for all property damage sustained by any one person in any one accident; and a limit of liability of not less than \$500,000 aggregate for any such damage sustained by two or more persons in any one accident.

This requirement for an Owner's protective policy shall be in addition to any and all other insurance requirements as set forth in the Contract Documents, if applicable.

END OF SECTION

SECTION 01090

REFERENCE STANDARDS

PART 1. GENERAL

1.1 REQUIREMENTS INCLUDED

- A. Applicability of Reference Standards.
- B. Provision of Reference Standards at site.
- C. Acronyms used in Contract Documents for Reference Standards.  
Source of Reference Standards.

1.2 QUALITY ASSURANCE

- A. For products or workmanship specified by association, trade, or Federal Standards, comply with requirements of the standard, except when more rigid requirements are specified or are required by applicable codes.
- B. The date of the standard is that in effect as of the Bid date, or date of Owner-Contractor Agreement when there are bids, except when a specific date is specified.
- C. When required by individual Specifications section, obtain copy of standard. Maintain copy at jobsite during submittals, planning, and progress of the specific work, until Substantial Completion.

1.3 SCHEDULE OF REFERENCES

- |        |   |
|--------|---|
| AASHTO | American Association of State Highway and<br>Transportation Officials<br>444 North Capitol Street, N.W.<br>Washington, DC 20001 |
| ACI    | American Concrete Institute<br>P.O. Box 19150<br>Reford Station<br>Detroit, MI 48219  |
| AGC    | Associated General Contractors of America<br>1957 E. Street, N.W.<br>Washington, DC 20006                                       |

AI Asphalt Institute  
Asphalt Institute Building  
College Park, MD 20740

AISC American Institute of Steel Construction  
400 North Michigan Avenue  
Eighth Floor  
Chicago, IL 60611

AISI American Iron and Steel Institute  
1000 16th Street, N.W.  
Washington, DC 20036

ANSI American National Standards Institute  
1430 Broadway  
New York, NY 10018

ASHRAE American Society of Heating, Refrigerating and  
Air Conditioning Engineers  
1791 Tullie Circle, N.E.  
Atlanta, GA 30329

ASME American Society of Mechanical Engineers  
345 East 47th Street  
New York, NY 10017

ASTM American Society for Testing and Materials  
1916 Race Street  
Philadelphia, PA 19103

AWWA American Water Works Association  
6666 West Quincy Avenue  
Denver, CO 80235

AWPA American Wood-Preservers Association  
7735 Old Georgetown Road  
Bethesda, MD 20014

AWS American Welding Society  
550 LeJeune Road  
Miami, FL 33135

CLFMI Chain Link Fence Manufacturers Institute  
1101 Connecticut Avenue, N.W.  
Washington, DC 20036

CRSI Concrete Reinforcing Steel Institute  
933 Plum Grove Road  
Schaumburg, IL 60195

EJCDC      Engineers Joint Contract Documents Committee  
American Consulting Engineers Council  
1050 15th Street, N.W.  
Washington, DC 20005

EJMA      Expansion Joint Manufacturers Association  
707 Westchester Avenue  
White Plains, NY 10604

FM          Factory Mutual System  
1151 Boston-Providence Turnpike  
Norwood, MA 02062

FS          Federal Specification  
General Services Administration  
Specifications and Consumer Information  
Distribution Section (WFSIS)  
Washington Navy Yard, Bldg. 197  
Washington, DC 20407

GA          Gypsum Association  
1603 Orrington Avenue  
Evanston, IL 60201

IEEE       Institute of Electrical and Electronics Engineers  
345 East 47th Street  
New York, NY 10017

IMI         International Masonry Institute  
815 15th Street, N.W.  
Washington, DC 20005

MIL         Military Specification  
Naval Publications and Forms Center  
5801 Tabor Avenue  
Philadelphia, PA 19120

ML/SFA     Metal Lath/Steel Framing Association  
221 North LaSalle Street  
Chicago, IL 60601

NAAMM     National Association of Architectural Metal  
Manufacturers  
221 North LaSalle Street  
Chicago, IL 60601

NEBB       National Environmental Balancing Bureau  
8224 Old Courthouse Road  
Vienna, VA 22180

NEMA National Electrical Manufacturers Association  
2101 L Street, N.W.  
Washington, DC 20037

NFPA National Forest Products Association  
1619 Massachusetts Avenue, N.W.  
Washington, DC 20036

NSWMA National Solid Waste Management Association  
1120 Connecticut Avenue, N.W.  
Washington, DC 20036

NTMA National Terrazzo and Mosaic Association  
3166 Des Plaines Avenue  
Des Plaines, IL 60018

PCA Portland Cement Association  
5420 Old Orchard Road  
Skokie, IL 60077

PCI Prestressed Concrete Institute  
201 North Wacker Drive  
Chicago, IL 60606

PS Product Standard  
U. S. Department of Commerce  
Washington, DC 20203

SDI Steel Deck Institute  
P.O. Box 3812  
St. Louis, MO 63122

SIGMA Sealed Insulating Glass Manufacturers Association  
111 East Wacker Drive  
Chicago, IL 60601

SJI Steel Joist Institute  
1703 Parham Road  
Suite 204  
Richmond, VA 23229

SMACNA Sheet Metal and Air Conditioning Contractors  
National Association  
8224 Old Court House Road  
Vienna, VA 22180

SSPC Steel Structures Painting Council  
4400 Fifth Avenue  
Pittsburgh, PA 15213

TAS Technical Aid Series  
Construction Specifications Institute  
601 North Madison Street  
Alexandria, VA 22314

TCA Tile Council of America, Inc.  
P.O. Box 326  
Princeton, NJ 08540

UL Underwriters Laboratories, Inc.  
333 Pfingston Road  
Northbrook, IL 60062

PART 2. PRODUCTS  
2.1 Not Used.  
PART 3. EXECUTION  
3.1 Not Used.

END OF SECTION

SECTION 01400

QUALITY CONTROL

PART 1. GENERAL

1.1 REQUIREMENTS INCLUDED

- A. General Quality Control.
- B. Workmanship.
- C. Manufacturers' Instructions.
- D. Manufacturers' Certificates.
- E. Mockups.
- F. Manufacturers' Field Services.
- G. Testing Laboratory Services.

1.2 RELATED REQUIREMENTS

- A. General Conditions: Inspection and testing required by governing authorities.
- B. Section 01090 - Reference Standards: Applicability of specified reference standards.
- C. Section 01300 - Submittals: Submittal of Manufacturers' Instructions.
- D. Section 03301 - Concrete Work: Tests required for concrete.

1.3 QUALITY CONTROL, GENERAL

- A. Maintain quality control over suppliers, manufacturers, products, services, site conditions, and workmanship, to produce work of specified quality.

1.4 WORKMANSHIP

- A. Comply with industry standards except when more restrictive tolerances or specified requirements indicate more rigid standards or more precise workmanship.
- B. Perform work by utilizing only persons qualified to produce workmanship of specified quality.
- C. Secure products in place with positive anchorage devices

designed and sized to withstand stresses, vibration, and racking.

#### 1.5 MANUFACTURERS' INSTRUCTIONS

- A. Comply with instructions in full detail, including each step in sequence. Should instructions conflict with Contract Documents, request clarification from A/E before proceeding.

#### 1.6 MANUFACTURERS' CERTIFICATES

- A. When required by individual Specification Sections, submit manufacturers' certificate, in duplicate, that products meet or exceed specified requirements.

#### 1.7 MOCKUPS

- A. When required by individual Specifications Section, erect complete, full-scale mockup of assembly at Project site. Tests will be performed in accordance with Section 01400, if applicable. Remove mockup at completion when approved by A/E.

#### 1.8 MANUFACTURER'S FIELD SERVICES

- A. When specified in respective Specification Sections, require supplier or manufacturer to provide qualified personnel to observe field conditions, conditions of surfaces and installation, quality of workmanship; start-up of equipment; test, adjust, and balance of equipment, as applicable; and, to make appropriate recommendations.
- B. Representative shall submit written report to A/E listing observations and recommendations.

#### 1.9 TESTING LABORATORY SERVICES

- A. Contractor shall employ and pay for services of an Independent Testing Laboratory to perform inspections, tests, and other services required by individual Specification Sections.
- B. Services will be performed in accordance with requirements of governing authorities or agencies and with specified standards.
- C. Reports will be submitted to A/E in duplicate giving observations and results of tests, indicating compliance

or non-compliance with specified standards and with Contract Documents.

D. Contractor shall cooperate with Testing Laboratory personnel; furnish tools, samples of materials, design mix, equipment, storage and assistance as requested.

1. Notify A/E and Testing Laboratory at least 48 hours prior to expected time for operations requiring testing services.

2. Make arrangements with Testing Laboratory and pay for additional samples and tests for Contractors' convenience.

PART 2. PRODUCTS

NOT USED

PART 3. EXECUTION

NOT USED

END OF SECTION

01400-3

SECTION 01568

EROSION CONTROL

PART 1. GENERAL

This work shall consist of erosion control on all cut and fill operations, excavation, backfill, or other construction activities within the limits of the construction site, within any temporary or permanent easements, and within any borrow site used during the period of construction. The protection of these sites shall continue throughout the construction period. During flood seasons, protect the sites by sandbagging, the pumping of water, and any other means appropriate to restrain flooding of plant and equipment. During dry weather, sprinkle the sites with water or use other means as necessary to provide dust control. In case of abnormally cold weather, any construction such as excavation work may be delayed until warmer weather or covered to prevent freezing.

All work shall be in accordance with the City of Spring Hill's National Pollution Discharge Elimination System (NPDES) Municipal Separate Storm Sewer System (MS4) Phase II Program. Prior to any excavation activities commencing, the developer, developer's engineer, and/or contractor shall apply for and receive an approved permit from the City of Spring Hill for such excavation activities. The application for permit will be reviewed by the Program Director and an approved permit shall be obtained prior to excavation activities. All erosion and sediment runoff control measures shall be installed in accordance with the approved permit and shall be maintained throughout the project cycle and until adequate and approved vegetative cover has been established. Erosion control measures such as mulching, silt fencing, check dams, or other applicable measures.

PART 2. PRODUCTS

Temporarily stabilize areas from which topsoil has been removed and topsoil stockpiles by seeding fast growing annuals such as rye and annual ryegrass, that provide quick protection. These annual grasses are to be seed certified by the State Department of Agriculture and can be worked into the soil when the site is prepared for final seeding of more permanent species. Use commercial lime and fertilizer on exposed areas, subject to severe erosion.

PART 3. EXECUTION

3.1 Conduct construction so as to provide the site with maximum protection from erosion at all times.

3.2 Conduct excavation activities to provide erosion and sediment control as follows:

3.2.1 Do not start clearing and excavation until a firm construction schedule is submitted to and approved by the City of Spring Hill. Continuously coordinate the schedule with the clearing and excavation activity.

3.2.2 In streets and other paved areas, remove excavated material from the site as construction progresses to prevent any erosion of this material.

3.2.3 In other areas, place the excavated material so as not to block any drainage area. Replace this excavated material in the trench immediately after repairs have been completed and are approved by the City of Spring Hill.

3.2.4 Retain natural vegetation whenever feasible. Install sediment control measures where needed and maintain throughout the project.

3.2.5 Restore and cover exposed areas subject to erosion as quickly as possible by means of seeding and mulching. Use diversion ditches or other methods as appropriate to prevent storm water from running over the exposed area until seeding is established as specified.

3.2.6 Take particular care along streams and drainage ditches so that fallen trees, debris, and excavated material will not adversely affect the streamflow. Exercise care to minimize the destruction of streambanks. Wherever the streambanks are affected by construction, reduce the slope of the streambanks to provide a suitable condition for vegetation protection. Minimize land exposure in terms of area and time.

3.2.7 Cover exposed excavated areas with mulch or vegetation.

3.2.8 Mechanically retard the rate of runoff water.

3.2.9 Trap the sediment contained in the runoff water utilizing approved sediment control measures.

3.2.10 Divert water from erosive areas.

3.2.11 Take care during the pouring of concrete, hauling of materials, etc., to keep vehicles from creating a severe erosion problem. Proper scheduling of operations and prompt repair of ruts created during this operation is necessary from this source.

3.2.12 Control dust by sprinkling or other means as necessary to keep it to a minimum.

3.2.13 Pave or otherwise stabilize roadways and driveways as soon as feasible.

3.2.14 Regrade and reseed surfaces eroded or otherwise damaged during any and all construction operations as necessary.

END OF SECTION

01568-3

SECTION 01620

STORAGE AND PROTECTION

PART 1. GENERAL

Not Used

PART 2. PRODUCTS

2.1 Not Used.

PART 3. EXECUTION

3.1 STORAGE, GENERAL

- A. Store products, immediately on delivery, in accordance with manufacturer's instructions, with seals and labels intact. Protect until installed.
- B. Arrange storage in a manner to provide access for maintenance of stored items and for inspection.

3.2 EXTERIOR STORAGE

- A. Provide substantial platforms, blocking, or skids, to support fabricated products above ground; slope to provide drainage. Protect products from soiling and staining.
- B. Store loose granular materials on clean, solid surfaces such as pavement, or on rigid sheet materials, to prevent mixing with foreign matter.
- C. Provide surface drainage to prevent erosion and ponding of water.

3.3 MAINTENANCE OF STORAGE

- A. Verify that surfaces of products exposed to the elements are not adversely affected; that any weathering of finishes is acceptable under requirements of Contract Documents.

END OF SECTION

SECTION 02221

UNCLASSIFIED EXCAVATION FOR UTILITIES

PART 1. GENERAL

1.1 The work called for by this section shall consist of clearing and grubbing, loosening, loading, removing, and disposing of, in the specified manner, all wet and dry materials (including rock) encountered that must be removed for construction purposes; furnishing, placing, and maintaining all sheeting, shoring, bracing, and timbering necessary for the proper protection and safety of the work; the workmen, the public, and adjacent property and improvements; the dewatering of trenches and other excavations; the preparation of satisfactory pipe beds; the backfilling and tamping of trenches, foundations, and other structures; the preparation of fills and embankments; the removal of unsuitable material from outside the normal limits of excavation and, where ordered by the A/E, their replacement with suitable materials; and all other grading or excavation work incidental to or necessary for the work. This work shall be performed as specified below.

PART 2. PRODUCTS

Not Used.

PART 3. EXECUTIONS

3.1 PREPARATION OF THE SITE

- A. Before starting construction, remove from the work site all vegetable growth (except as hereinafter excluded), debris, and/or other objectionable matter as well as any buildings and/or other structures that the drawings and/or the A/E specifically indicate are to be removed. Dispose of this refuse material in a manner acceptable to the A/E.
- B. In certain areas it may be desirable for existing trees, shrubs, or other vegetation on the site to be preserved for the permanent landscape. Such vegetation may be shown on the drawings, specifically listed in the specifications, marked on the site, or identified by the A/E. In no case damage or remove such growth without written permission from the Owner.

0100-252  
02221-1

- C. If the area to be excavated is occupied by trees, brush, or other vegetable growth, clear such growth, grub the excavated area, and remove all large roots to a depth of not less than 2 feet below the bottom of the proposed construction. Dispose of the growth removed in a manner satisfactory to the A/E. Fill all holes or cavities created during this work that extend below the subgrade elevation with suitable material, and compact to the same density as the surrounding material.
- D. Trees, cultivated shrubs, etc., that are situated within public rights-of-way and/or construction easements through private property but not directly within the excavation area shall remain undisturbed unless it is necessary to remove them so that the work can be performed safely and unless their removal is specifically ordered by the A/E. Take special precautions to protect and preserve such growth throughout all stages of the construction.
- E. Preparation of the site shall be considered an integral part of the excavation and one for which no separate payment shall be allowed.

### 3.2 UNSUITABLE MATERIALS

- A. Wherever muck, quicksand, soft clay, swampy ground, or other material unsuitable for foundations, subgrade, or backfilling is encountered, remove it and continue excavation until suitable material is encountered. The material removed shall be disposed of in the manner described below. Then refill the areas excavated for this reason with compacted 4-inch lifts of crushed stone up to the level of the lines, grades, and/or cross sections shown on the drawings. The top 6 inches of this refill shall be No. 67 (TDOT) crushed stone for bedding.

### 3.3 ROCKS AND BOULDERS

- A. Any material that is encountered within the limits of the required excavation that cannot be removed except by drilling and/or blasting, including rock, boulders, masonry, hard pan, chert, shale, street and sidewalk pavements, and/or similar materials, shall be considered as unclassified excavation, and no separate payment will be made therefore.
- B. Should rock be encountered in the excavation, remove it by blasting or otherwise. Where blasts are made, cover the excavation with enough excavation material and/or timber

or steel matting to prevent danger to life and property. The Contractor shall secure, at his own expense, all permits required by law for blasting operations and the additional hazard insurance required. Observe all applicable laws and ordinances pertaining to blasting operations.

- C. Excavate rock over the horizontal limits of excavation and to a depth of not less than 6 inches below the bottom of pipe up to 30 inches in diameter and not less than 12 inches below the bottom of larger pipes if rock extends to such depth. Then backfill the space below grade with No. 67 (TDOT) crushed stone or other approved material, tamp to the proper grade, and make ready for construction. For monolithic concrete sewers and for structures, excavate rock to the outside bottom of the structure or sewer.

#### 3.4 DISPOSAL OF MATERIALS

- A. Whenever practicable, all materials removed by excavation that are suitable for backfilling pipe trenches or for other purposes shown on the drawings or directed by the A/E shall be used for these purposes. Any materials not so used shall be considered waste materials and disposed of by the Contractor as specified below.
- B. Waste materials may be deposited in spoil areas at locations approved by the A/E. Do not leave in unsightly piles but instead spread in uniform layers, neatly level, and shape to drain. Seed as specified in Section 02485, Seeding.
- C. Once any part of the work is completed, properly dispose of all surplus or unused materials (including waste materials) left within the construction limits of that work. Leave the surface of the work in a neat and workman like condition, as described below.
- D. The disposal of waste materials shall be considered an integral part of the excavation work and one for which no separate payment shall be allowed.

#### 3.5 EXCAVATION FOR TRENCHES, MANHOLES, AND STRUCTURES

- A. Unclassified excavation for pipelines shall consist of the excavation necessary for the construction of water, sewer, and other pipes and their appurtenances (including manholes, inlets, outlets, headwalls, collars, concrete saddles, and pipe protection) that are called for by the drawings. It shall include clearing and grubbing where

necessary, backfilling and tamping pipe trenches and around structures, and disposing of waste materials, all of which shall conform to the applicable provisions set forth elsewhere in these specifications.

- B. The Contractor may, if he chooses, use a motor powered trenching machine. If he does, however, he shall be fully responsible for the preservation or repair of existing utility service connections.
- C. Unless the construction of lines by tunneling, jacking, or boring is called for by the drawings or specifically authorized by the A/E, make excavation for pipelines in open cut and true to the lines and grades shown on the drawings or established by the A/E on the ground. Cut the banks of trenches between vertical parallel planes equidistant from the pipe centerline. The horizontal distance between the vertical planes (or, if sheeting is used, between the inside faces of that sheeting) shall vary with the size of the pipe to be installed, but shall not be more than the distance determined by the following formula:  $4/3d + 15$  inches, where "d" represents the internal diameter of the pipe in inches. When approved in writing by the A/E, the banks of trenches from the ground surface down to a depth not closer than 1 foot above the top of the pipe may be excavated to non-vertical and nonparallel planes, provided the excavation below that depth is made with vertical and parallel sides equidistant from the pipe centerline in accordance with the formula given above. Any cut made in excess of the formula  $4/3d + 15$  inches shall be at the expense of the Contractor and may be cause for the A/E to require that stronger pipe and/or a higher class of bedding be used at no cost to the Owner.
- D. For rigid pipe, shape the bottom of all trenches to provide uniform bearing for the bottom of the pipe barrel. For plastic sewer lines, provide a minimum of 6 inches of No. 67 (TDOT) crushed stone for bedding.
- E. Excavate bell holes for bell and spigot pipe at proper intervals so that the barrel of the pipe will rest for its entire length upon the bottom of the trench. Bell holes shall be large enough to permit proper jointing of the pipe. Do not excavate bell holes more than 2 joints ahead of pipe laying.

- F. Excavation for manholes, inlets, and other incidental structures shall not be greater in horizontal area than that required to allow a 2 foot clearance between the outer surface of the structure and the walls of the adjacent excavation or of the sheeting used to protect it. The bottom of the excavation shall be true to the required shape and elevation shown on the drawings. No earth backfilling will be permitted under manholes, inlets, headwalls, or similar structures. Should the Contractor excavate below the elevations shown or specified, he shall, at his own expense, fill the void with either concrete or granular material approved by the A/E.
- G. Do not excavate pipe trenches more than 200 feet ahead of the pipe laying, and perform all work so as to cause the least possible inconvenience to the public. Construct temporary bridges or crossings when and where the A/E deems necessary to maintain vehicular or pedestrian traffic.
- H. In all cases where materials are deposited along open trenches, place them so that in the event of rain no damage will result to the work and/or to adjacent property.
- I. Excavation for other structures may be performed with non-vertical banks except beneath pavements or adjoining existing improvements. Do not permit the horizontal area of the excavation to exceed that required to allow a 2 foot clearance between the outer surface of the structure and the banks of the excavation or the sheeting used to protect the embankments. The bottom of the excavation shall be true to the required shape and elevation shown on the drawings.

### 3.6 SHEETING, SHORING, AND BRACING

- A. Take special care to avoid damage wherever excavation is being done. Sufficiently sheet, shore, and brace the sides of all excavations to prevent slides, cave-ins, settlement, or movement of the banks and to maintain the specified trench widths. Use solid sheets in wet, saturated, or flowing ground. All sheeting, shoring, and bracing shall have enough strength and rigidity to withstand the pressures exerted, to keep the walls of the excavation properly in place, and to protect all persons

and property from injury or damage. Separate payment will not be made for sheeting, shoring, and bracing, which are considered an incidental part of the excavation work.

- B. Wherever employees may be exposed to moving ground or cave-ins, shore and lay back exposed earth excavation surfaces more than 5 feet high to a stable slope, or else provide some equivalent means of protection. Effectively protect trenches less than 5 feet deep when examination of the ground indicates hazardous ground movement may be expected. Guard the walls and faces of all excavations in which employees are exposed to danger from moving ground by a shoring system, sloping of the ground, or some equivalent protection.
- C. Comply with all OSHA standards in determining where and in what manner sheeting, shoring, and bracing are to be done. The sheeting, shoring, and bracing system shall be designed by a professional engineer licensed in the State of Tennessee and shall be subject to approval by the A/E. However, such approval does not relieve the Contractor of the sole responsibility for the safety of all employees, the effectiveness of the system, and any damages or injuries resulting from the lack or inadequacy of sheeting, shoring, and bracing.
- D. Where excavations are made adjacent to existing buildings or structures or in paved streets or alleys, take particular care to sheet, shore, and brace the sides of the excavation so as to prevent any undermining of or settlement beneath such structures or pavement. Underpin adjacent structures wherever necessary, with the approval of the A/E.
- E. Do not leave sheeting, shoring, or bracing materials in place unless this is called for by the drawings, ordered by the A/E, or deemed necessary or advisable for the safety or protection of the new or existing work or features. Remove these materials in such a manner that the new structure or any existing structures or property, whether public or private, will not be endangered or damaged and that cave-ins and slides are avoided.
- F. Fill and compact all holes and voids left in the work by the removal of sheeting, shoring, or bracing as specified herein.

- G. The Contractor may use a trench box, which is a pre-fabricated movable trench shield composed of steel plates welded to a heavy steel frame. The trench box shall be designed to provide protection equal to or greater than that of an appropriate shoring system.

### 3.7 THE DEWATERING OF EXCAVATION

- A. Provide and keep in operation enough suitable pumping equipment whenever necessary or whenever directed to do so by the A/E. Give special attention to excavations for those structures that, prior to proper backfilling, are subject to flotation from hydrostatic uplift.

### 3.8 BORROW EXCAVATION

- A. Whenever the backfill of excavated areas or the placement of embankments requires more material than is available from authorized excavations, or whenever the backfill material from such excavations is unsuitable, then obtain additional material from other sources. This may require the opening of borrow pits at points accessible to the work. In such cases, make suitable arrangements with the property owner and pay all incidental costs, including any royalties, for the use of the borrowed material. Before a borrow pit is opened, the quality and suitability of its material shall be approved by the A/E. All state and local regulations concerning borrow pits, drainage and erosion control shall be strictly followed.
- B. Excavate borrow pits in such a way that the remaining surfaces and slopes are reasonably smooth and that adequate drainage is provided over the entire area. Construct drainage ditches wherever necessary to provide outlets for water to the nearest natural channel, thus preventing the formation of pools in the pit area. Leave the sides of borrow pit cuts at a maximum slope of 2:1 unless otherwise directed by the A/E.
- C. Properly clear and grub borrow pits, and remove all objectionable matter from the borrow pit material before placing it in the backfill.
- D. The taking of materials from borrow pits for use in the construction of backfill, fills, or embankments shall be considered an incidental part of the work; no separate payment shall be made for this.

### 3.9 BACKFILLING

- A. Begin backfilling after the line construction is completed and then inspected and approved by the A/E. Trenches 10-foot deep or less shall contain a 6-inch stone envelope around the pipe. Trenches greater than 10-foot deep shall contain 6-inches of stone under the pipe and on each side with 6-inches of stone placed above the pipe.
- B. Backfill material above the pipe envelopes shall consist either of fine, loose earth like sandy soil or loam or of granular material that is free from clods, vegetable matter, debris, stone, and/or objectionable materials and that has a size of no more than 6-inches. Place this backfill simultaneously on either side of the trench in even layers that before compaction are no more than 6 inches deep. Thoroughly and completely tamp each layer into place before placing additional layers. When shown on the drawings, this backfill shall, at locations beneath or closely adjacent to pavement, consist of No. 67 (TDOT) crushed stone. Compaction of backfill material layers shall be at 98% by standard proctor test. Where adjacent to and within paved areas the top 12-inches of the trench at subgrade shall consist of crusher-run stone compacted at 98% by standard proctor test. Compaction testing shall be at intervals directed by the site inspector.
- C. From 1 foot above the pipe upward, the backfill material may contain broken stones that make up approximately 3/4 of the backfill total volume. However, if this type of backfill is used, there must be enough spalls and earth materials to fill all voids completely. The maximum dimension of individual stones in such backfill shall not exceed 6 inches, and the backfill material shall be placed and spread in even layers not more than 12 inches deep. At locations beneath or closely adjacent to pavement or at locations of improvements subject to damage by displacement, tamp and thoroughly compact the backfill in layers that, before compaction, are 6 inches deep. In other areas, the backfill for the upper portion of the trenches may be placed without tamping but shall be compacted to a density equivalent to that of adjacent earth material as determined by laboratory tests. Use special care to prevent the operation of backfilling equipment from causing any damage to the pipe.

- 
- 
- 
- 
- D. If earth material for backfill is, in the opinion of the A/E, too dry to allow thorough compaction, then add enough water so that the backfill can be properly compacted. Do not place earth material that the A/E considers too wet or otherwise unsuitable.
- E. Wherever excavation has been made within easements across private property, the top 1 foot of backfill material shall consist of fine loose earth free from large clods, vegetable matter, debris, stone, and/or other objectionable materials.
- F. Wherever trenches have been cut across or along existing pavement, temporarily pave the backfill of such trenches by placing Class A, Grade D, crushed stone as the top 12 inches of the backfill. Maintain this temporary pavement either until the permanent pavement is restored or until the project is accepted by the Owner. On heavily traveled roadways, cold mix or leveling course binder 4-inches thick shall be installed and maintained until permanent pavement is installed.
- G. Conduct backfilling around manholes, inlets, outfalls, and/or structures in the same manner as specified above for pipelines except that even greater care is necessary to prevent damage to the utility structure.
- H. Wherever pipes have diameters of 15 inches or less, do not use power operated tampers to tamp that portion of the backfill around the pipe within 1 foot above the pipe.
- I. Perform backfilling so as not to disturb or injure any pipe and/or structure against which the backfill is being placed. If any pipe or structure is damaged and/or displaced during backfilling, open up the backfill and make whatever repairs are necessary, whenever directed to do so by the A/E.
- J. Backfilling and clean-up operations shall closely follow pipe laying; failure to comply with this provision will result in the A/E's requiring that the Contractor's other activities be suspended until backfilling and clean-up operations catch up with pipe laying.
- K. Compaction Requirements: Unless specified otherwise elsewhere, under buildings and 2 times the depth of pipe beyond, and under roads and 2 times the depth beyond the shoulder, compact to 95% maximum density in accordance with ASTM D698. In all other locations, compact to 90% maximum density.

3.10 MAINTENANCE

- A. Seed and maintain in good condition all excavated areas, trenches, fills, embankments, and channels until final acceptance by the Owner.
- B. Maintain trench backfill at the approximate level of the original ground surface by periodically adding backfill material wherever necessary and whenever directed to do so by the A/E. Continue such maintenance until final acceptance of the project, or until the A/E issues a written release.

3.11 SLOPES

- A. Neatly trim all open cut slopes, and finish to conform either with the slope lines shown on the drawings or the directions of the A/E. Leave the finished surfaces of bottom and sides in reasonably smooth and uniform planes like those normally obtainable with hand tools, though the Contractor will not be required to use hand methods if he is able to obtain the required degree of evenness with mechanical equipment. Conduct grading operations so that material is not removed or loosened beyond the required slope.

END OF SECTION

SECTION 02485

SEEDING

PART 1. GENERAL

1.1 This work shall be performed in all disturbed areas not receiving such site improvements as buildings, roads, walks, sod, planting, etc., and shall include, but not necessarily be limited to, all seed bed preparation; the supplying and placing of soil additives, seed, and mulch wherever required by the drawings or directed by the A/E; and maintenance.

1.2 Unless otherwise approved in writing by the A/E, seeding operations shall be limited to the following planting periods:

- A. Spring - March 1 through May 30
- B. Fall - August 15 through October 31

1.3 Refer to other sections for items affecting seeding. Coordinate this work with that specified by other sections for timely execution.

PART 2. PRODUCTS

2.1 GRASS SEED: Kentucky 31 Fescue (*Festuca elatior*) and/or annual rye meeting the requirements of the State Department of Agriculture and furnished in new bags or bags that are sound and not mended; no "below standard" seed will be accepted.

2.2 FERTILIZER: commercially manufactured; Grade 10-10-10; furnished in standard containers that are clearly marked with the name, weight, and guaranteed analysis of the contents and that ensure proper protection in transportation and handling; and in compliance with all local, state, and federal fertilizer laws.

2.3 AGRICULTURAL LIMESTONE: containing a minimum of 85% calcium carbonate and magnesium carbonate combined, 85% of which passes a No. 10 mesh sieve.

2.4 MULCH: stalks of rye, oats, wheat, or other approved grain crops properly cured prior to baling, air dried, and reasonably free of noxious weeds and weed seeds or other material detrimental to plant growth.

PART 3. EXECUTION

3.1 Perform all seeding and related work as a continuous operation. Sow seed as soon as the seed bed has been prepared, and perform subsequent work in a continuous manner.

3.2 Before beginning seeding operations in any area, complete the placing of topsoil and final grading, and have the work approved by the A/E.

3.3 Scarify, disk, harrow, rake, or otherwise work each area to be seeded until the soil has been loosened and pulverized to a depth of not less than 2 inches. Perform this work only when the soil is in a tillable and workable condition.

3.4 Apply fertilizer and agricultural limestone uniformly over the seed bed, and lightly harrow, rake, or otherwise incorporate them into the soil for a depth of approximately 1 inch at the following rates:

Fertilizer: 15 pounds per 1,000 square feet

Agricultural Limestone: 40 pounds per 1,000 square feet

3.5 Sow seed uniformly with a rotary seeder, wheelbarrow seeder, hydraulic equipment or by other satisfactory means.

3.6 The seeding rate shall be 5 pounds per 1,000 square feet for Kentucky 31 Fescue (*Festuca elatior*).

3.7 When seeding during March 1 through April 1 and October 1 through November 20, add an additional 3 pounds per 1,000 square feet of annual rye grass.

3.8 Perform no seeding during windy weather or when the ground surface is frozen, wet, or otherwise untillable.

3.9 Spread mulch material evenly over the seeded areas immediately following the seeding operation.

Mulch Rate: 2 bales (100 pound minimum) per 1,000 square feet

3.10 The mulch rate may be varied by the A/E, depending on the texture and condition of the mulch material and the characteristics of the area seeded. Cover all portions of the seeded areas with a uniform layer of mulch so that approximately 25% of the ground is visible.

3.11 No equipment, material storage, construction traffic, etc., will be permitted on newly seeded ground.

3.12 Dispose of all surplus materials as directed by the Owner.

#### PART 4. INSPECTIONS

The A/E shall inspect the seeding within 60 days after planting and determine if it is acceptable.

#### PART 5. GUARANTEE

5.1 Secure an acceptable growth of grass in all areas designated for seeding.

5.2 An area is considered acceptable if it is represented by a minimum of 100 seedlings per square foot of the permanent species of grass representative of the seed mixture. If an acceptable growth is not obtained on the first planting, reseeding and remulching will be required.

5.3 If the planting is less than 50% successful, rework the ground, refertilize, reseed, and remulch.

END OF SECTION

02485-3

SECTION 02575

PAVEMENT REPAIR

PART 1. GENERAL

1.1 The work specified by this section shall consist of repairing or replacing all damaged pavement, whether public or private. Dirt shoulders, roads, streets, drives, and walks are to be restored to their original condition as an incidental part of the installation of utilities. Repair damaged base on either side of a trench wherever necessary. Trim the oxidation surface to neat lines outside of the trench wall, and repave the entire area as specified below and as shown on the drawings or on the standard drawings.

1.2 Both these specifications and the drawings make reference to the current edition of the standard specifications of the Tennessee Department of Transportation (TDOT). Even though the weather limitations, construction methods, and materials specifications contained in the TDOT specifications may not be explicitly repeated in these specifications, they shall, wherever applicable to the work called for by this section, be considered as implied and therefore adhered to. However, the various subsections "Basis for Payment" contained in the TDOT specifications shall not be considered applicable.

- A. Refer to other sections for work related to that covered by this section.

PART 2. PRODUCTS

2.1 MINERAL AGGREGATE BASE: Class A, Grading D crushed stone (TDOT specifications, Section 303, subsection 903.05)

2.2 BITUMINOUS PRIME COATS: cutback asphalt, Grade RC-250, or emulsified asphalt, Grade AE-P (Section 402, Subsections 904.02 and 904.03)

2.3 CRUSHED STONE CHIPS: Size 6 or Size 7 (Subsection 903.14)

2.4 DOUBLE BITUMINOUS SURFACE: for both courses, either cutback asphalt, Grade RC-800 or RC-3000, or emulsified asphalt, Grade RS-2 (Subsections 904.02 and 904.03)

2.5 ASPHALTIC CONCRETE BINDER: Grading B or C, as directed by the A/E (Section 307)

2.6 BITUMINOUS TACK COAT: Grade AE-3 (Section 403, Subsection 904.03)

2.7 ASPHALTIC CONCRETE SURFACE: Grading E (Section 411)

2.8 QUICK DRY TRAFFIC MARKING PAINT (WHITE AND YELLOW):  
Subsection 910.05.

### PART 3. EXECUTION

#### 3.1 SUBGRADE

- A. Before any base material is installed, compact the subgrade of the area to be paved to 98% of optimum density as determined by ASTM D698 (Standard Proctor).
- B. The backfill material shall contain no topsoil or organic matter. For all areas where subgrade has been prepared, test for uniformity of support by driving a loaded full size dump truck at a speed of 2 to 3 mph over the entire surface. Make further improvements on all areas that show a deflection of 1 inch or more. When completed, the finished subgrade shall be hard, smooth, stable, and constructed in reasonably close conformance with the lines and grades that existed prior to beginning construction.
- C. When a base course is compacted, cut back the surface course of the existing pavement a minimum of 1 foot beyond the limit of the joint between the old and new base course or as shown on the standard drawings. Take special care to ensure good compaction of the new base course at the joint. Apply and compact the surface to conform to the existing pavement with no surface irregularity.

#### 3.2 BASE

- A. Install a mineral aggregate base in accordance with the City of Spring Hill's approved roadway classification standard drawings. The maximum compacted thickness of any one layer shall be 4-inches and the total thickness of the base shall be that indicated by the standard drawings or as shown on the plans.

#### 3.3 ASPHALTIC CONCRETE BINDER

- A. Apply a bituminous prime coat of emulsified asphalt, Grade AE-P, or cutback asphalt, Grade RC-250, at a rate of 0.38 to 0.42 gallon per square yard. Take care to prevent the bituminous material's splashing on exposed faces of curbs and gutters, walls, walks, trees, etc; if such splashing does occur, remove it immediately. After the prime coat has been properly cured, apply an asphaltic concrete binder to the thickness shown on the City of Spring Hill's approved roadway section drawings.

- B. Carefully place the material to avoid segregation of the mix. Broadcasting of the material will not be permitted. Remove any lumps that do not readily break down.

### 3.5 ASPHALTIC CONCRETE SURFACE

- A. If the asphaltic concrete surface course is to be placed directly on the mineral aggregate base, place a bituminous prime coat as described above. If, however, the surface course is to be placed on a binder course, then apply a bituminous tack coat of the sort specified above under products at a rate of 0.05 to 0.10 gallon per square yard. Take care to prevent the bituminous material's splashing on exposed faces of curbs, gutters, walls, walks, trees, etc.; if such splashing does occur, remove it immediately. After the prime or tack coat has been properly cured, apply the asphaltic concrete to the thickness shown of the drawings or standard drawings. Apply the surface course as described above for the binder course.

### 3.7 SMOOTHNESS

- A. The finished surfaces shall conform to the lines and grades that existed prior to construction. No deviations, variations, or irregularities exceeding 1/4 inch in any direction when tested with a 12 foot straightedge will be permitted in the finished work, nor will any depressions that will not drain. Correct all such defects.

### 3.8 SAMPLING AND TESTING

- A. Submit to the A/E test reports made by an independent testing laboratory on the crushed stone aggregate, bituminous materials, and asphaltic concrete design mixes, and obtain his approval of these reports before starting paving operations.
- B. Tests shall be made of the completed elements of the pavement to ascertain the compacted thickness of the base and surface courses. If sections with deficient thicknesses are found, the full section for a reasonable distance on each side of the deficiency shall be refused. Remove and reinstall all such sections. Patch all test holes in connection with thickness tests.
- C. When making surface tests, furnish one man to mark all surface defects for corrections.

END OF SECTION

## SECTION 02600

### MANHOLES

#### PART 1. GENERAL

1.1 Manholes shall be precast or monolithic concrete with eccentric cones unless otherwise approved by the A/E. All manholes shall contain Zypex add mixture to be batch mixed with the concrete prior to casting of the manhole. Upon requesting, and approval of the City of Spring Hill Sewer Department Director and A/E, Zypex may be added to the manhole after it has been cast but prior to leaving the manufacturer's site.

1.2 Refer to other sections for items affecting manholes. Coordinate this work with that specified by other sections for timely execution.

1.3 Shop drawings are required for castings, plastic gaskets, and precast manholes specified in this section.

#### PART 2. PRODUCTS

2.1 CONCRETE MASONRY: reinforced or plain, meeting the applicable requirements of Section 03303, Concrete for Utility Lines.

2.2 CASTING ADJUSTMENT: Only concrete grade rings only will be allowed to adjust the casting elevation.

2.3 MORTAR: composed of one (1) part portland cement and two (2) parts sand (volumetric measure) thoroughly mixed in a tight box, with water added gradually and mixed continually until mortar has attained the proper consistency for use in brick masonry; prepared only in such quantities as needed for immediate use; mortar mixed for more than 30 minutes, retempered, or previously set will not be allowed.

2.4 GRAY IRON CASTINGS: cast iron conforming to the requirements of Class 30, ASTM A48; made accurately to the required dimensions; sound, smooth, clean, and free from blisters and other defects; not plugged or otherwise treated to remedy defects; machined so that covers rest securely in the frames with no rocking and are in contact with frame flanges for the entire perimeter of the contact surfaces; thoroughly cleaned subsequent to machining and, before rusting begins, painted with a bituminous coating so as to present a smooth finish; tough and tenacious when cold, but not tacky and

with no tendency to scale; and with the actual weight in pounds stenciled or printed by the manufacturer on each casting in white paint.

2.5 PLASTIC GASKET FOR PRECAST MANHOLES: Preformed plastic gasket shall meet or exceed all requirements of FS SS-S-00210, "Sealing Compound, Preformed Plastic for Pipe Joints," Type I, rope form. The sealing compound shall be produced from blends of refined hydrocarbon resins and plasticizing compounds reinforced with inert mineral filler and shall contain no solvents, irritating fumes, or obnoxious odors. The compound shall not depend on oxidizing, evaporating, or chemical action for its adhesive or cohesive strength. It shall be supplied in extruded rope form of suitable cross section and in such sizes as to seal the joint space when the pipes are laid. Use two (2) complete ropes at each joint. The sealing compound shall be protected by a suitable removable two (2) piece wrapper, which shall be designed so that half may be removed longitudinally without disturbing the other half in order to facilitate application of the sealing compound. The flexible plastic gasket shall also meet the requirements of the following table:

| <u>Composition</u>                  | <u>Test Method</u> | Minimum | Maximum |
|-------------------------------------|--------------------|---------|---------|
| Bitumen (Petroleum Plastic Content) | ASTM D4            | 50      | 70      |
| Ash Inert Mineral Matter            | AASHO T111         | 30      | 50      |
| Volatile Matter                     | ASTM D6            | --      | 2.0     |

| <u>Property</u>                              | <u>Test Method</u> | Minimum       | Maximum |
|--|--------------------|---------------|---------|
| Specific Gravity at 77 degrees F             | ASTM D71           | 1.20          | 1.30    |
| Ductility at 77 degrees F (cm)               | ASTM D113          | 5.0           | ---     |
| Softening Point                              | ASTM D36           | 320 degrees F | ---     |
| Penetration at 77 degrees F (150 gms) 5 sec. | ASTM D217          | 50            | 120     |

2.6 LADDER BARS: an aluminum alloy weighing 2.2 pounds or 1/2 inch steel reinforced rod encapsulated in polypropylene plastic.

2.7 MATERIAL TESTING: All precast reinforced concrete manhole risers and tops specified herein shall be tested and inspected by a commercial testing laboratory approved by the A/E prior to delivery to the site, and all materials that fail to conform to these specifications shall be rejected. After delivery to the site, any materials that have been damaged in transit or are otherwise

unsuitable for use in the work shall be rejected and removed from the site. Supply certified copies in duplicate of the inspection and acceptance reports of the testing laboratory to the A/E before using the materials. The commercial testing laboratory shall be engaged and paid for by the Contractor. Submit a certificate from the manufacturer of the castings indicating that they meet all applicable requirements of these specifications.

### PART 3. EXECUTION

3.1 Dewater sufficiently to maintain the ground water level at or below the bottom of the manhole foundation prior to and during placement of the foundation. All manhole installations require the subgrade soil to be compacted to a minimum of 98% standard proctor density and a minimum of 12" of TDOT #67 compacted base stone.

3.2 Obtain an adequate foundation for all manhole structures by removing and replacing unsuitable material with well graded granular material, by tightening with coarse rock, or by such other means as provided for foundation preparation of the connected sewers or as directed by the A/E. Wherever water is encountered at the site, place all cast-in-place bases or monolithic structures on a one-piece waterproof membrane to prevent any movement of water into the fresh concrete.

3.3 When the foundation subgrade has been prepared and is approved by the Town, carefully construct the concrete foundation for monolithic manholes to the line and grade required by the drawings. Construct the manholes after the concrete foundation has been allowed to set for a period of not less than 24 hours.

3.4 For precast manholes, carefully block the base section above the prepared surface so that it is fully and uniformly supported in true alignment; make sure that all entering pipe can be inserted at proper grade. Then place the concrete foundation and invert under and upon this base section as shown in the standard drawings. A base section with monolithic foundation (bottom) may be used when approved by the A/E.

3.5 Thoroughly wet and then completely fill all lift holes and joints, inside and outside, with non-shrink grout to ensure water tightness.

3.6 Construct monolithic concrete manholes and bases of 4,000 psi concrete in accordance with the provisions of this section and applicable provisions of Section 03303, Concrete for Utility Lines. The ladder bars shall be cast in place.

3.7 Carefully set the cast iron frame for the cover at the required elevation, and properly bond it to the masonry with cement grout and mastic seal. The required elevation is defined as the top of casting elevation on the approved construction plans. Whenever manholes are constructed in paved areas, tilt the top surface of the frame and cover so as to conform to the exact slope, crown, and grade of the existing adjacent pavement.

3.8 Manhole inverts shall be constructed of concrete or Portland cement mortared masonry fill and may, at the Contractor's option, be covered with cement mortar to the approximate cross section of the sewers connected to them. Make any necessary changes in cross sections gradually from side to side of the manhole; make changes in direction of flow of the sewers to a true curve of as large a radius as is permitted by the size of the manhole.

3.9 All rigid unreinforced pipe entering or leaving the manhole shall be provided with flexible joints within 12 inches of the manhole structure, or encase the full joint in concrete. Place such pipe on firmly compacted bedding, particularly in the area of the manhole excavation, which is normally deeper than excavation for sewer trenches. Take special care to see that the openings through which pipes enter the structures are completely and firmly rammed full of shrink proof mortar or otherwise constructed to ensure watertightness.

3.10 A flexible pipe to manhole connector shall be used to provide a watertight joint between the gravity sewer line and manhole. This connector shall be Kor-N-Seal I Connector or an approved equal.

3.11 Where the difference in the invert elevation of two or more lines intersecting in one manhole is 24 inches or more, construct a drop manhole. Drop manholes shall be similar in construction to standard manholes except that a drop connection of pipe and fittings of the proper sizes and materials shall be constructed outside the manhole and supported by 3,000 psi concrete as indicated by the standard drawings.

3.12 Place backfill by hand around the manhole and to a distance of at least one (1) pipe length into each trench, and tamp with selected material up to an elevation of 12 inches above the crown of all entering pipes. Continue backfilling in accordance with the requirements for trench backfilling.

3.13 Each manhole shall be vacuum tested immediately after installation or rehabilitation and prior to backfilling. No standing water shall be allowed in the manhole excavation which may

affect the accuracy of the test. All lifting holes and exterior joints shall be filled and pointed with an approved non-shrink mortar. All pipes and other openings into the manhole shall be suitably plugged in such a manner as to prevent displacement of the plugs while the vacuum is drawn. Installation and operation of the vacuum equipment and indicating devices shall be in accordance with equipment specification and instructions provided by the manufacturer. A vacuum of 10 inches shall be drawn. The time for the vacuum to drop to 9.0 inches for one minute shall be recorded.

Acceptance for four (4) feet diameter manholes shall be defined as when the time to drop one (1) inch meets 60 minutes.

For manholes five (5) feet in diameter, add an additional 15 seconds. For manholes six (6) feet in diameter, add an additional 30 seconds. If the manhole fails the test, necessary repairs shall be made and the vacuum test repeated until the manhole passes the test. If the manhole joint mastic or gasket is displaced during the vacuum test, the manhole shall be disassembled, the seal replaced, and the manhole re-tested.

END OF SECTION

SECTION 02722

SANITARY SEWERS (GRAVITY)

PART 1. GENERAL

1.1 Pipe material for sewer lines 18 inches and smaller shall be SDR 26 PVC. Class 250 Ductile Iron Pipe shall be used at a depth greater than 20-feet and within fill materials. All piping must meet the stone bedding, encapsulation and deflection requirements of ASTM for the pipe material, size, backfill material and soil conditions.

1.2 Pipe material for sewer lines 21 inches and larger shall be SDR 26 PVC (or equivalent ASTM 679 PS 115). Class 250 Ductile Iron Pipe shall be used at depths greater than 20-feet and within fill materials. All piping must meet the stone bedding, encapsulation and deflection requirements of ASTM for the pipe material, size, backfill material and soil conditions.

1.3 Shop drawings are required for all products specified in this section.

1.4 Refer to other sections for items affecting gravity sewers. Coordinate this work with that specified by others sections for timely execution.

PART 2. PRODUCTS

2.1 PIPE

- A. Polyvinyl Chloride (PVC): to meet and/or exceed the requirements of ASTM D3034, SDR 26; suitable for use as a gravity sewer conduit with provisions for contraction and expansion at each joint; with a rubber ring and standard length 12.5 feet plus or minus one (1) inch; designed to pass all tests at 73 degrees F (plus or minus 3 degrees F); six (6) inches long sections of pipe to be subjected to impact from a free falling type (20 pounds, Type A) in accordance with ASTM D2444 with no evident splitting or shattering (denting not considered a failure); and with a minimum envelope of six (6) inches of granular material around the pipe, but with all other bedding and backfilling requirements remaining the same as for other pipe material.

- B. Ductile Iron: with push-on joints conforming to ASTM A746, minimum Class 250 thickness unless.
- C. Lateral Branches: to be tees of the same material as the main sewer and have a four (4) inch inside diameter for residential and six (6) inch diameter for commercial unless otherwise specified or noted; able to withstand all test pressures involved without leakage.

## 2.2 JOINTS AND JOINTING MATERIALS

- A. All rubber end rings shall be extruded or molded and cured such that any cross section will be dense, homogenous and free of parasites, blisters, pitting, and other imperfections. The basic rubber material, EPDM, shall meet ASTM C443 with the exception of 40-60 duro hardness. The resilient interlocked end seals shall be duro A-40-70, plus or minus 2.
- B. Polyvinyl Chloride (PVC) Pipe Joints: Joints for sewer plastic pipe shall meet all requirements of ASTM D3212 standard specifications. Joint design shall be tested and certified to result in no leakage under prescribed laboratory test conditions of joint alignment, load conditions, pressure and vacuum, and deflection. Pipe and fittings shall have integral bell with elastomeric seal joint.
- C. Ductile Iron Pipe Joints: gasket type joints for bell and spigot ductile iron pipe designed to meet the infiltration requirements of these specifications; jointing to comply with ANSI A21.11.

## 2.3 COMPRESSION COUPLINGS

- A. When dissimilar pipe materials like PVC are joined, use compression couplings that are resistant to the corrosive action of soils and sewage and that will provide a permanent watertight joint. The compression couplings shall be of natural or synthetic rubber or rubber-like material and shall comply with the requirements and test methods specified in Table 2 of ASTM C425. The coupling shall meet the leak requirements specified in ASTM C425, and the bands for attaching the couplings to the dissimilar pipes shall be of stainless steel meeting ASTM A167 or A240. Each coupling shall bear the manufacturer's identifying mark and an indication of its size.

PART 3. EXECUTION

3.1 PIPE LAYING

- A. Lay no pipe except in the presence of an inspector representing the City.
- B. Before placing sewer pipe in position in the trench, carefully prepare the bottom and sides of the trench, and install any necessary bracing and sheeting as provided in Section 02221, Unclassified Excavation for Utilities.
- C. Wherever necessary to provide satisfactory bearing surface, place concrete cradles as shown on the drawings or as directed by the A/E. Cradles shall be of concrete and conform to the dimensions shown on the drawings. Concrete placed outside the dimensions shown shall be at the Contractor's expense.
- D. Install piping utilizing a laser set at the correct design slope. Set reference points for both line and grade at each manhole. Where grades are 0.6% or less, check the elevation of the beam each 100 feet with an offset point or engineer's level.
- F. Do not allow water to run or stand in the trench while pipe laying is in progress or before the trench has been backfilled. Do not at any time open up more trench than the available pumping facilities are able to dewater.
- G. Correct trench bottoms found to be unsuitable for foundations after pipe laying operations have started, bringing them to exact line and grade with compacted stone as necessary and as approved by the City of Spring Hill's Sewer Director and A/E.
- H. Carefully inspect each piece of pipe and special fitting before it is placed, and lay no defective pipe in the trench. Pipe laying shall proceed upgrade, starting at the lower end of the grade and with the bells upgrade. When pipe laying is not in progress, keep the ends of the pipe tightly closed with an approved temporary plug.
- I. Bell holes shall be large enough to allow ample room for the pipe joints to be properly made. Cut out bell holes no more than two (2) joints ahead of the pipe laying. Carefully grade the bottom of the trench between bell holes so that each pipe barrel rests on a solid foundation for its entire length. Lay each pipe joint so as to form a close concentric joint with adjoining pipe and to avoid sudden offsets or inequalities in the flow line.

- J. Before constructing or placing any joints, demonstrate to the A/E, by completing at least one sample joint, that the methods to be used conform to the specifications and will provide a watertight joint and further that the workmen to be involved in this phase of work are thoroughly familiar and experienced with the type of joint proposed.
- K. No other type of joint may be used unless authorized in writing by the A/E.
- L. Install tee branches in sewer lines to serve properly each lot facing or abutting on the street or alley in which sewer is being laid and at such other locations as may be designated by the A/E. If tee branches are not to be used immediately, close them with approved stoppers that are held in place to prevent infiltration and withstand all test requirements. All service line end caps shall be marked by a green metal fence post as to allow the builder to determine the exact location of the service lateral.
- M. For all tees that are plugged and laid in rock, blast a minimum of six (6) linear feet of ditch line in the direction and to the approximate grade of the future lateral as directed by the A/E, but do not excavate the material. This shall be done at no extra cost to the Owner. Furnish the A/E with a record of the exact location of each tee installed.
- N. If the work consists of constructing a new sewer to replace an existing one, connect existing service lines to the new line.
- O. New service laterals shall conform to the standard drawings.
- P. The Contractor shall provide an above-ground green metal fence post marker at the property line to indicate the termination of new service laterals.
- Q. As the work progresses, thoroughly clean the interior of the pipe in place. After each line of pipe has been laid, carefully inspect it, and remove all earth, trash, rags, and other foreign matter from its interior.
- R. After the joints have been completed, they shall be inspected, tested, and accepted by the A/E before being

covered. The pipe shall meet the test requirements for water tightness; immediately repair any leak or defect discovered at any time after completion of the work. Any pipe that has been disturbed after joints were formed shall be taken up, the joints cleaned and remade, and the pipe relayed at the Contractor's expense. Carefully protect all pipe in place from damage until backfilling operations are completed.

- S. Do not begin the backfilling of trenches until the pipe in place has been inspected and approved by the A/E.
- T. Lay sewers at least ten (10) feet horizontally from any existing or proposed water main. If this is not practical, the sewer may be laid closer than ten (10) feet to a water main provided it is laid in a separate trench and the elevation of the top of the sewer is at least 18 inches below the bottom of the water main.
- U. Where a sewer crosses under water mains, the top of the sewer shall be at least 18 inches below the bottom of the water main. If the elevation of the sewer cannot be varied to meet the above requirements, relocate the water main to provide this separation, or else reconstruct it with mechanical joint ductile iron pipe for a distance of ten (10) feet on each side of the sewer with a full joint of the water main centered over the sewer.
- V. If it is impossible to obtain proper horizontal and vertical separation as stipulated above, construct both the water main and the sewer of mechanical joint ductile iron pipe, and pressure test each.
- W. Perform boring by means of auguring to the size, line, and grade shown on the drawings. Jack the steel casing pipe into place as the boring proceeds. Weld sections of casing pipe together to provide a watertight joint.
- X. Make connections to all existing sewer lines as shown on the drawings or as directed by the A/E. Make connections either by removing a section of the sewer from the existing line and inserting a wye or tee branch of the proper size or by constructing a manhole, junction box, regulator chamber, or other structure as shown on the drawings.
- Y. Make connections to existing manholes or inlets by methods of machine coring and installation of a Kor-N-Seal boot connector. After the boot connector has been

properly installed in the existing structure, insert a length of sewer pipe into the boot connector and tighten the band strap of the boot connector. Fill around the void area between the pipe and existing structure located on the inside of the existing structure with non-shrink grout to a neat finish. Shape or reshape the existing inverts or bottom of the manhole/structure as necessary to fit the invert of the sewer pipe and allow unobstructed flow through the existing structure.

- Z. Joint dissimilar pipe by using suitable compression couplings. If compression couplings are not available, make jointing with a special fabricated coupling approved by the A/E.
- AA. Provide concrete protection or concrete cap as shown on the drawings for pipe sewers that, when completed, have less than 2.5 feet of covering in non-traffic areas and four (4) feet of cover in traffic areas. If such protection is not shown on the drawings, place it in accordance with the typical section shown.
- BB. Carefully protect from damage all existing sewers, water lines, gas lines, sidewalks, curbs, gutters, pavements, electrical lines, and other utilities or structures in the vicinity of the work at all times. If it is necessary to repair, remove, and/or replace any such utility or structure in order to complete the work properly, do so in compliance with the provisions set forth in other section of these specifications. Any such work shall be considered incidental to the construction of pipe sewers, and no additional payment will be allowed therefore.
- CC. Water service connections will be repaired or replaced by the Contractor at his expense as an incidental part of the work.
- DD. Service or house connections to existing sewers that are damaged or removed shall be repaired or replaced by the Contractor at his own expense as an incidental part of the work.
- EE. For PVC and ductile iron pipe, furnish a certificate from the pipe manufacturer indicating that the pipe meets all applicable requirements of these specifications.
- FF. All piping must meet the stone bedding, encapsulation and

deflection requirements of ASTM for the pipe material, size, backfill material and soil conditions. The minimum pipe stiffness for PVC pipe at 5% deflection shall be 46 for all sizes when tested in accordance with ASTM D2412; external loading properties of plastic pipe shall be by parallel plate loading.

GG. A specimen of PVC pipe six (6) inches long shall be flattened between parallel plates in a suitable press until the distance between the plates is 40% of the outside diameter of the pipe. The rate of loading shall be uniform and such that the compression is complete in two (2) to five (5) minutes.

HH. After being immersed for two (2) hours in a sealed container of anhydrous acetone (99.5% pure), a sample ring of PVC pipe shall show no visible spalling or cracking when tested in accordance with ASTM D2152 (swelling or softening is not considered a failure).

II. The Contractor shall provide a concrete check dam in the trench for the gravity sewer lines. The check dam shall be constructed in accordance with the detail included in the Standard Drawings. The check dam shall be provided for all gravity sewer lines. The maximum spacing of the walls shall be 400 feet. The check dam shall be installed at a distance of one pipe length upstream of each manhole and on each creek bank, swale etc. as to prevent ground water from traveling along the trench and to prevent surface water from entering the trench at creek crossings.

### 3.2 TESTING OF GRAVITY SEWERS

#### A. Visual Tests

1. Upon completion of the construction or earlier if the A/E deems advisable, the A/E will make a visual inspection of the sewer and construction site. Immediately repair all leaks and defects found by such inspection.
2. In addition to general cleanup and leakage, the following standards shall be used to determine failure or defects of this project.
3. Sewers shall be built so as to remain true to line and grade. The inclining grade of the bottom of the sewer after completion shall be such that, after flooding, the flood water drains off so that no remaining puddle of water is deeper than 1/2

inch on pipe 36 inches internal diameter or smaller and 3/4 inch on pipe larger than 36 inches internal diameter. Any section of pipe that does not comply with the specifications at any time previous to final acceptance of the work shall be replaced or relayed at the Contractor's expense.

4. The Contractor will be held strictly responsible that all parts of the work bear the load of the backfill. If cracks 1/100 inch develop in the pipe within one (1) year from the date of final acceptance of the work, the Contractor will be required to replace, at his expense, all such cracked pipe. To this end, the Contractor is advised to purchase pipe under a guarantee from the manufacturer, guaranteeing proper service of sewer pipe under conditions established by the drawings, specifications, and local conditioning at the site of the work.

B. Air Testing for Sewers 24 Inches and Smaller

1. Perform low pressure air testing as follows:
  - a. Furnish all equipment, facilities, and personnel necessary to conduct the test. The test shall be observed by a representative of the A/E.
  - b. Make the air test after all services have been installed and backfilling has been completed and compacted.
  - c. Perform the first series of air tests after 2,000 linear feet but before 4,000 linear feet of sewer has been laid. The purpose of this first series of tests is to assure both the Contractor and the A/E that the materials and methods of installation meet the intent of these specifications. Conduct the remainder of the tests after approximately each 10,000 linear feet has been laid.
  - d. Plug all tees and ends of sewer services with flexible joint plugs or caps securely fastened to withstand the internal test pressures. Such plugs or caps shall be readily removable, and

their removal shall provide a socket suitable for making a flexible jointed lateral connection or extension.

- e. Prior to testing, check the pipe to see that it is clean. If not, clean it by passing a full-gauge squeegee through the pipe. It shall be the Contractor's responsibility to have the pipe cleaned.
- f. Immediately following this check or cleaning, test the pipe installation with low pressure air. Supply the air slowly to the plugged pipe installation until the internal air pressure reaches 4.0 psi more than the average back pressure of any ground water that may submerge the pipe. Allow at least two (2) minutes for temperature stabilization.
- g. The pipeline shall be considered acceptable when tested at an average pressure of 3.0 psi more than the average back pressure of any ground water that may submerge the pipe, if the section under test does not lose air at a rate greater than 0.0015 cfm per square foot of internal pipe surface area. Air pressure drop from a stabilized pressure of 4 to 3 psi for one minute more than the average back pressure of any ground water that may submerge the pipe.
- h. The requirements of this specification shall be considered satisfied if the time required in seconds for the pressure to decrease from 4.0 to 3.0 psi more than the average back pressure of any ground water that may submerge the pipe is not less than that shown in the following table:

ALLOWABLE AIR LOSS VALUES PER 100 LF

| <u>Pipe Size</u> | <u>Time (Seconds)</u> |
|------------------|-----------------------|
| 6 inches         | 42                    |
| 8 inches         | 60                    |
| 10 inches        | 60                    |
| 12 inches        | 60                    |
| 15 inches        | 60                    |
| 18 inches        | 60                    |
| 21 inches        | 60                    |
| ≥ 24 inches      | 60                    |

- i. If the pipe installation fails to meet these requirements, the Contractor shall determine at his own expense the source or sources of leakage and repair or replace all defective materials or workmanship. The completed pipe installation shall meet the requirements of this test before being considered acceptable.
  2. The recommended procedures for conducting acceptance tests are as follows:
    - a. Clean pipe that is to be tested.
    - b. Plug all pipe outlets with suitable test plugs, and brace each plug securely.
    - c. Increase gauge pressure in the test by the amount of ground water pressure at the crown of the pipe.
    - d. Add air slowly to the portion of the pipe installation being tested until the internal air pressure is raised to 4.0 psi more than the average back pressure above the crown of the pipe.
    - e. After the above internal pressure is obtained, allow at least two (2) minutes for air temperature to stabilize, adding only the amount of air required to maintain pressure.
    - f. After two (2) minutes, disconnect the air supply.
    - g. When pressure decreases to 4.0 psig either by leaking down or by bleeding down with a release valve, start the stopwatch, and determine the time in seconds that is required for the internal air pressure to reach 3.0 psig. Compare this time interval as calculated above. If the time is more than that calculated, the test shall be assumed to be acceptable.
  3. Plugs used to close the sewer pipe for the air test must be securely braced to prevent the unintentional release of a plug, which can become a high velocity projectile. Locate gauges, air piping manifolds, and valves at the top of the ground. No one shall be permitted to enter a manhole where a plugged pipe is under pressure. Four pounds air pressure (gauge) develops a force against the plug in a 12 inch pipe

of approximately 450 pounds. Pipes more than 30 inches in diameter shall not be air tested because of the difficulty of adequately blocking the plugs. Provide a safety release device set to release at ten (10) psi between the air supply and the sewer under test.

4. Regardless of the outcome of the tests, repair any noticeable leak.

C. Testing for Sewers Larger than 24 Inches

1. Using Existing High Ground Water

- a. Where the natural ground water is 24 inches or more above the top of a section of pipe, measure the flow of water in the pipe and the rates of seepage and infiltration. Measure the flow rate by using a calibrated weir. Leave the weir in the line until the flow rate has stabilized. The Contractor is responsible for verifying the ground water level by providing sight gauges in manholes or digging test holes at suitable locations.
- b. The total seepage and infiltration of ground water as determined by the test shall in no case exceed 25 gallons per 24 hours per inch-mile of pipe. Make infiltration tests on all sewer construction before placing the lines in service and before making any connections to other sewers. If the amount of infiltration into the sewer(s) is in excess of the maximum quantity specified above, then re-caulk or remake the joints, relay the sewer (if necessary), or perform other remedial construction, at the Contractor's expense, in order to reduce ground water infiltration to within the specified limits.
- c. In making infiltration tests, furnish the required equipment and labor and do the necessary pumping under the direction of the A/E. Tests must be repeated until each sewer individually meets the specifications for infiltration amounts as set out above.

## 2. Exfiltration Test

- a. Where the ground water is not 24 inches or more above the top of the pipe section being tested then perform an exfiltration test. Bulkhead the pipe below the lower manhole of the section being tested with a pneumatic plug or other device. Insert a vent pipe 48 inches long in the stopper of the upper end of that section. Then fill the lower manhole with water, or add water until there is a minimum of four (4) feet over the upper end; make certain that all air is forced out through the vent tube. Measure the drop in the level of the water in the manhole due to exfiltration over a specific time, and calculate the water loss due to exfiltration. The total exfiltration shall not exceed that specified above for infiltration. Conditions encountered in construction may vary this procedure slightly, but essentially this is the method to be used.

## 3. Repairs

- a. Regardless of the outcome of any tests, repair any noticeable leak.

### 3.3 VISUAL INSPECTION OF MISCELLANEOUS MATERIALS

- A. All material used on this project will be visually inspected by the A/E at the site for conformance to the required specifications. When reasonable doubt exists that said material meets the specifications, the A/E may require certified mill tests, samples, and/or tests by an independent laboratory or other suitable form of verification that the material meets the required specifications.

### 3.4 DEFLECTION TESTING FOR PVC PIPE

- A. Test deflection of the pipe by passing a 9-arm pin go/no-go mandrel sized to 95% of the pipe diameter of the actual pipe used with the pipe in place and covered. Make this acceptance test after backfill consolidation has occurred.

### 3.5 CLEANUP

- A. After completing each section of the sewer line, remove all debris, construction materials, and equipment from

the site of the work, grade and smooth over the surface on both sides of the line, and leave the entire area in a clean, neat, and serviceable condition.

### 3.6 VIDEO INSPECTION

A. New gravity sewer lines shall be required to be inspected using CATV video inspection equipment. The City of Spring Hill will require the contractor to perform this type of inspection to determine if debris or defects exist within the sewer line. This video inspection shall be performed after all utilities have been installed, roadways paved and all other infrastructure installed and completed. This inspection shall serve to verify that sewer lines, manholes and service laterals are clean and free of debris and defects. Any defects discovered during the video inspection shall be corrected in accordance with these standard specifications at the cost of the developer and/or his contractor.

END OF SECTION

SECTION 02724

SEWAGE FORCE MAIN

PART 1. GENERAL

1.1 Furnish all material, equipment, tools, and labor in connection with the sewage force main, complete and in accordance with the drawings and these specifications.

1.2 It shall be the Contractor's responsibility to ensure that all necessary materials are furnished to him and that those found to be defective in manufacture are replaced at no extra cost to the Owner. Materials damaged in handling after being delivered by the manufacturer shall be replaced at the Contractor's own expense. If installed material is found to be defective before the final acceptance of the work, the cost of both the material and labor needed to replace it shall not be passed on to the Owner.

1.3 The Contractor shall be responsible for safely storing materials needed for the work that have been accepted by him until they have been incorporated into the completed project. Keep the interiors of all pipes, fittings, and other accessories free from dirt and foreign matter at all times.

1.4 Refer to other sections for work related to that specified by this section. Coordinate this work with that required by other sections for timely execution.

1.5 Minimum force main size shall be four (4) inches in diameter.

PART 2. PRODUCTS

2.1 Ductile Iron Pipe and Fittings

2.1.1 Ductile iron pipe shall be made of good quality ductile cast iron that meets the requirements of ASTM E8-61T. The pipe shall be centrifugally cast in metal or sand-lined molds. It shall be made and tested in accordance with ASTM A536 and be subjected to and able to withstand a hydrostatic pressure of 500 psi.

2.1.2 The pipe shall be plain end ductile iron pipe with a push-on single gasket joint and shall conform to ANSI A21.51/AWWA C151. The design thickness shall be Class 50 for pipe as defined by ASTM A21.50/AWWA C150.

2.1.3 The length of each individual piece of ductile iron pipe shipped must be plainly marked on that piece of pipe.

2.1.4 The push-on single gasket joints shall be UL approved and able to withstand an operating pressure of 200 psi.

2.1.5 The bell of each pipe shall have a tapered annular opening and a cast or machined retaining groove for the gasket. The gasket groove shall have a flared design so that maximum deflection will be provided. The plain spigot end of the pipe shall be beveled in order to simplify its entry into and centering within the bell and the compression of the gasket.

2.1.6 The gasket shall be of high quality vulcanized rubber made in the form of a solid ring to exact dimensions. The design of the gasket groove in the bell of the pipe and the design, hardness, and other properties of the gasket itself shall be such that the joint is liquidtight for all pressures from a vacuum to the maximum internal liquid pressure of 350 psi.

2.1.7 Enough lubricant shall be furnished with each order to provide a thin coat on the spigot end of each pipe. This lubricant shall be nontoxic, impart no taste or smell, and have no harmful effect on the rubber gasket. It shall have a consistency that will allow it to be easily applied to the pipe in either hot or cold weather and that will enable it to adhere to either wet or dry pipe.

2.1.8 Standard and special fittings shall be ductile iron. Use standard mechanical joint fittings unless otherwise shown on the drawings. All fittings shall conform to ANSI A21. 10/AWWA C110.

2.1.9 Pipe and pipe fittings shall have cement linings as specified in ANSI A21. 4/AWWA C104. In addition, a bituminous seal coat or asphalt emulsion spray coat approximately 1 mil thick shall be applied to the cement lining in accordance with the pipe manufacturer's standard practices.

## 2.2 PVC Pipe

2.2.1 All plastic pipe shall be made from Class 12454-B polyvinyl chloride plastic (PVC 1120) as defined by ASTM D1784.

2.2.2 All Class 200 pipe shall have NSF approval and be manufactured in accordance with ASTM D2241. The following tests shall be run for each machine on each size and type of pipe being produced, as specified below:

2.2.2.1 Flattening Test: once per shift in accordance with ASTM D2412. Upon completion of the test, the specimen shall not be split, cracked, or broken.

2.2.2.2 Acetone Test (Extrusion Quality Test): once per shift in accordance with ASTM D2152. There shall be no flaking, peeling, cracking, or visible deterioration on the inside or outside surface after completion of the tests.

2.2.2.3 Quick Burst Test: once per 24 hours in accordance with ASTM 5199.

| <u>SDR</u> | <u>Pressure Rating</u> | <u>Minimum Bursting Pressure, psi</u> |
|------------|------------------------|---------------------------------------|
| 13.5       | 315                    | 1,200                                 |
| 17         | 250                    | 1,000                                 |
| 21         | 200                    | 800                                   |

2.2.2.4 Impact Tests: for 6" and larger, once per shift in accordance with ASTM D2444; for 4" and smaller, once each 2 hours in accordance with ASTM D2444.

2.2.2.5 Wall Thickness and Outside Dimensions Tests: once per hour in accordance with ASTM D2122.

2.2.2.6 Bell Dimensions Test: once per hour in accordance with ASTM D3139.

2.2.3 If any specimen fails to meet any of the above mentioned tests, all pipe of that size and type manufactured between the test periods must be scrapped and a full set of tests rerun.

2.2.4 Furnish a certificate from the pipe manufacturer stating that he is fully competent to manufacture PVC pipe of uniform texture and strength and in full compliance with these specifications and further stating that he has manufactured such pipe and done so in sufficient quantities to be certain that it will meet all normal field conditions. In addition, the manufacturer's equipment and quality control facilities must be adequate to ensure that each extrusion of pipe is uniform in texture, dimensions, and strength. Also furnish a certificate from the manufacturer certifying that the pipe furnished for this project meets the requirements of these specifications.

2.2.5 All pipe shall be manufactured in the United States of America. All pipe for any one project shall be made by the same manufacturer.

2.2.6 Pipe 8" and larger shall be furnished in 20 feet lengths. The Contractor's methods of storing and handling the pipe shall be approved by the A/E. All pipe shall be supported within five (5) of each end; in between the end supports, there shall be additional supports at least every 15 feet. The pipe shall be stored away from heat or direct sunlight. The practice of stringing pipes out along the proposed force main routes will not be allowed.

2.2.7 Certain information shall be applied to each piece of pipe. At the least, this shall consist of:

- 2.2.7.1 Nominal size
- 2.2.7.2 Type of material
- 2.2.7.3 SDR or class
- 2.2.7.4 Manufacturer
- 2.2.7.5 NSF Seal of Approval

2.2.8 Pipe that fails to comply with the requirements set forth in these specifications shall be rejected.

2.2.9 The pipe shall have push-on joints designed with grooves in which continuous molded rubber ring gaskets can be placed. Gaskets shall be made of vulcanized natural or synthetic rubber; no reclaimed rubber will be allowed. The gaskets shall be of the manufacturer's standard design dimensions and of such size and shape as to provide a positive seal under all combinations of joint and gasket tolerance. The gasket and annular groove shall be designed and shaped so that when the joint is assembled, the gasket will be radially compressed to the pipe and locked in place against displacement, thus forming a positive seal.

2.2.10 The spigot end of each pipe shall be beveled so that it can be easily inserted into the gasket joint, which in turn shall be designed so that the spigot end may move in the socket as the pipe expands or contracts. The spigot end shall be striped to indicate the distance into which it is to be inserted into the socket. Each joint shall be able to accommodate the thermal expansions and contractions experienced with a temperature shift of at least 75 degrees F.

2.2.11 Enough lubricant shall be furnished with each order to provide a coat on the spigot end of each pipe. This lubricant shall be nontoxic, impart no taste or smell, have no harmful effect on the gasket or pipe material, and support no bacterial growth. The lubricant containers shall be labeled with the manufacturer's name.

2.2.12 Joints shall be manufactured in accordance with ASTM D3139 except that the thickness of the bell shall be, as a minimum, equal to that of the barrel. Joints shall be either integral bell and ring joints with rubber compression gaskets as manufactured by the Clow Corporation, Johns-Manville, or Vulcan Plastic Corporation; twin gasket couplings as manufactured by the Certain-Teed Products Corporation; or equal. However, the pipe and bell must be made by the same manufacturer.

2.2.13 Standard and special fittings shall be ductile iron. Use standard mechanical joint fittings. All fittings shall conform to the specifications of ANSI A21. 10/AWWA C110. The gaskets shall be ducked tipped transition gaskets for use with PVC pipe.

2.2.14 Fittings shall be lined with a thin cement lining as specified in ANSI A21. 4/AWWA C104; this lining is to be furnished at no extra cost. In addition, a bituminous seal coat or asphalt emulsion spray coat approximately 1 mil thick shall be applied to the cement lining in accordance with the pipe manufacturer's standard practices.

2.2.15 Fitting laying lengths shall conform to ANSI A21. 10/AWWA C110.

2.2.16 Fittings shall be in accordance with the standard mechanical joint fittings manufactured by the U.S. Pipe and Foundry Company, American Cast Iron Pipe Company, Clow Corporation, or equal.

### PART 3. EXECUTION

#### 3.1 Installation of Force Main

3.1.1 Lay the force main to and keep it at the lines and grades required by the drawings. All fittings shall be at the required locations, and spigots well centered in the bells. Where the grades are 0.2% or less, either use batterboards or a laser to maintain the required slopes.

3.1.2 Unless otherwise indicated by the drawings, all force mains shall have at least 36 inches of cover. The pipe shall slope continuously between high and low points and have a minimum of 60" cover at the high points. No departure from this policy shall be made except at the order of the A/E, or unless shown otherwise on the drawings.

3.1.3 Provide and use tools and facilities that are satisfactory to the A/E and that will allow the work to be done in a safe and convenient manner. Use a derrick, ropes, or other suitable equipment to lower all pipe and fittings into the trench one piece at a time. Carefully lower each piece so that neither it nor any protective coating or lining it may have will be damaged. Under no circumstances, drop or dump force main materials into the trench.

3.1.4 Lower no pipes and fittings into the trench until they have been swabbed to remove any mud, debris, etc., that may have

accumulated within them. After the pipe has been lowered, remove all unnecessary materials from it. Before any pipe is laid, brush and wipe clean the outside of its spigot end and the inside of its bell and ensure that the pipe is dry and oil-free.

3.1.5 Take every precaution to keep foreign material from getting into the pipe while it is being placed in the trench. If the crew laying the pipe cannot put it into the trench and in place without allowing earth to get inside it, then place a heavy, tightly woven canvas bag of suitable size over each end of the pipe and leave it there until it is time to connect that pipe to the one adjacent to it.

3.1.6 Place no debris, tools, clothing, or other materials in the pipe during laying operations.

3.1.7 After a length of pipe has been placed in the trench, center the spigot end in the bell of the adjacent pipe, and then insert to the depth specified by the manufacturer and bring to the correct line and grade. Secure the pipe in place by tamping an approved backfill material around it.

3.1.8 Bell holes shall be big enough so that there is ample room for the pipe joints to be properly made. Between bell holes, carefully grade the bottom of the trench so that each pipe barrel will rest on a solid foundation for its entire length.

3.1.9 Whenever pipe laying is not in progress, close the open ends of pipe in the trench with a water tight plug or by other means approved by the A/E. Caulk the joints of any pipe in the trench that cannot be completed until a later time with packing in order to make them as watertight as possible; this shall be done not only at the end of each working day but also before work is stopped for lunch periods, bad weather, or any other reason. If there is water in a trench, this seal shall remain in place until the trench has been pumped completely dry.

3.1.10 The cutting of pipe so that fittings or closure pieces can be inserted shall be done in a neat and workmanlike manner and without any damage to the pipe. Follow the manufacturer's recommendations concerning how to cut and machine the ends of the pipe in order to leave a smooth end at right angles to the pipe's axis.

3.1.11 The flame cutting of pipe by means of an oxyacetylene torch will not be allowed.

3.1.12 Unless otherwise directed by the A/E, lay pipe with the bell ends facing in the direction of laying.

3.1.13 Wherever pipe must be deflected from a straight line (in either the vertical or horizontal plane) in order to avoid obstructions or plumb stems, or wherever long radius curves are permitted, the amount of deflection shall not exceed that necessary for the joint to be satisfactorily made, nor that recommended by the pipe manufacturer, and shall be approved by the A/E.

3.1.14 Lay no pipe in water or when it is the A/E's opinion that trench conditions are unsuitable. If crushed stone is used to improve trench conditions or as backfill for bedding the pipe, this shall be considered incidental to the project, and no separate payment will be made for its use.

3.1.15 Install thrust blocks wherever the force main changes direction (e.g., at tees and bends), at dead ends, or at any other point where the manufacturer recommends and/or the A/E indicates that they are to be used.

3.1.16 Make all joints, whether standard mechanical or push-on joints, in conformance with the recommendations of the joint manufacturer as approved by the A/E.

3.1.17 For detection purposes, a 14 gage solid strand copper tracing wire (shielded) and an approved metallic tape identified as "sewer" shall be installed as per the manufacturer's instructions. Connections between wires shall be soldered or connected with wire nut fasteners and wrapped.

## 3.2 Hydrostatic Tests

### 3.2.1 Pressure Test

3.2.1.1 After pipe has been laid and backfilled as specified above, subject all newly laid pipe or any valved section thereof to a pressure of 200 psi. All connections (if applicable) are to be laid prior to testing the main and tested as part of the test of the main.

3.2.1.2 The duration of each pressure test shall be at least one (1) hour.

3.2.1.3 Slowly fill each valved section of pipe with water, and apply the specified test pressure (based on the elevation of the lowest point of the line or section under test and corrected to the elevation of the test gauge) with a pump connected to the pipe in a manner satisfactory to the A/E. Furnish the water, pump, pipe, connections, gauges, and all necessary apparatus.

3.2.1.4 Before applying the specified test pressure, expel all air from the pipe. If air/vacuum assemblies are not available at high places, make the necessary taps at the points of highest elevation before testing, and insert plugs after the test has been completed.

3.2.1.5 Carefully examine all exposed pipes, fittings, and valves, during the test. Remove any cracked or defective pipes, fittings, and/or valves, discovered in consequence of this pressure test, and replace with sound material in the manner specified. Repeat the test until the results are satisfactory to the A/E.

### 3.2.2 Leakage Test

3.2.2.1 Conduct the leakage test after the pressure test has been satisfactorily completed. Furnish the water, pump, pipe, connections, gauges, measuring devices, and all other necessary apparatus as well as all necessary assistance to conduct the test.

3.2.2.2 The duration of each leakage test shall be two (2) hours; during the test, subject the main to a pressure of 150 psi.

3.2.2.3 Leakage is defined as the amount of water which must be supplied to the newly laid pipe or any valved section in order to maintain the specified leakage test pressure after the pipe has been filled with water and the air expelled.

3.2.2.4 No pipe installation will be accepted until the leakage is less than the number of gallons per two (2) hour period listed below:

| <u>Pipe Sizes</u> | <u>Gallons per 1,000 Feet<br/>of Pipe</u> |
|-------------------|---|
| 2" - 2-1/4"       | 0.2                                       |
| 3"                | 0.5                                       |
| 4"                | 0.6                                       |
| 6"                | 0.9                                       |
| 8"                | 1.2                                       |
| 10"               | 1.5                                       |
| 12"               | 1.9                                       |
| 14"               | 2.2                                       |
| 16"               | 2.6                                       |
| 18"               | 2.9                                       |
| 20"               | 3.2                                       |
| 24"               | 3.8                                       |

3.2.2.5 Should any test of pipe laid disclose leakage greater than that specified, the Contractor shall, at his own expense, locate and repair the defective joints until the leakage is within the specified allowance.

### 3.3 Cleanup

After completing each section of force main, remove all debris and all construction materials and equipment from the work site. Then grade and smooth over the surface on both sides of the main. The entire area shall be clean and left in a condition satisfactory to the A/E. Seed and mulch as required elsewhere in these specifications.

END OF SECTION

SECTION 02725

BORING AND CASING FOR SANITARY SEWERS

PART 1. GENERAL

1.1 The work to be performed hereunder shall consist of the installation of casing pipe and carrier pipe for water lines as shown on the drawings or as called for in these specifications. For the open cut casing pipes, it shall include the excavation of the trench, placing proper bedding material, furnishing and installing the casing pipe, furnishing and installing the carrier pipe, backfilling, and disposing of the excess excavated materials. For the boring and jacking of casing pipes, it shall include the excavation of a boring pit, auger boring between the point as specified on the drawings, furnishing and installing of the carrier pipe, and disposing of the excavated materials in the manner herein provided.

PART 2. PRODUCTS

2.1 CASING PIPE

A. The casing pipe shall be of steel meeting the latest approved American Railway Engineering Association "Specifications" for Pipelines for Carrying Flammable and Nonflammable Substances." The steel casing pipe shall have a minimum yield strength of 35,000 PSI and shall have the minimum wall thickness shown in the following table:

TABLE OF MINIMUM WALL THICKNESS FOR STEEL CASING PIPE  
FOR E72 LOADING

| <u>Carrier Pipe Diameter</u> | <u>Casing Pipe Diameter</u> | <u>Nominal Thickness</u> |
|------------------------------|-----------------------------|--------------------------|
| 4 inches                     | 8 inches                    | 0.250 inches             |
| 6 inches                     | 12 inches                   | 0.250 inches             |
| 8 inches                     | 16 inches                   | 0.312 inches             |
| 10 inches                    | 20 inches                   | 0.312 inches             |
| 12 inches                    | 22 inches                   | 0.312 inches             |
| 14 inches                    | 24 inches                   | 0.344 inches             |
| 16 inches                    | 26 inches                   | 0.375 inches             |
| 18 inches                    | 28 inches                   | 0.406 inches             |

B. When the casing pipe is installed without benefit of a protective coating, the wall thickness shown above shall be increased to the nearest standard size, which is a minimum of 0.063 inches greater than the thickness shown.

2.2 CARRIER PIPE: The carrier pipe shall be either Class 250 Ductile Iron Pipe or Class 200 PVC pipe.

### PART 3. EXECUTION

#### 3.1 BORING

- A. The boring shall be accomplished by means of auguring to the size, line and grade shown on the drawings.

#### 3.2 INSTALLATION OF CASING PIPE

- A. For open cut of casing pipes, install the steel casing pipe into the open cut as the trench excavation proceeds. Weld sections of casing pipe together to provide watertight joints, and replace the protective coatings in areas where it is damaged by welding.
- B. For boring casing pipes, jack the steel casing pipe into place as the boring proceeds. Weld sections of casing pipe together to provide watertight joints.
- C. Do not remove unacceptable casing without prior approval from the A/E. If the removal of casing pipe is permitted, make proper provisions to prevent caving in of the earth surrounding the casing.

#### 3.3 INSTALLATION OF CARRIER PIPE

- A. The carrier pipe shall be furnished by the Contractor. Upon acceptance of the casing, install the carrier pipe in the casing by jacking it through the casing. Casing spacers and end caps are required.

#### 3.4 LAYOUT OF WORK

- A. The Owner will provide the necessary control points required by the Contractor for this construction. The Contractor will provide the detailed layout required to keep the excavation and pipe installation on grade.

### 4. GUARANTEE OF WORK

4.1 Guarantee a usable completed casing between the points specified and to the line and grade specified. The allowable

tolerance at the downstream end point of the casing shall be such that the invert of the carrier pipe may be positioned within a vertical area limited on the top by an elevation no higher than the elevation shown on the drawings and on the bottom by an elevation no lower than the existing inlet pipe invert.

4.2 The allowable tolerance at the upstream end point of the casing shall be such that the invert of the carrier pipe may be positioned at the elevation shown on the drawings.

END OF SECTION

02725-3

CONCRETE FOR UTILITY LINES

PART 1. GENERAL

1.1 This item shall include furnishing and installing concrete blocking, cradles, anchors, caps, pipe protection, and/or encasement at the locations shown on the drawings and/or directed by Spring Hill's representative.

PART 2. PRODUCTS

Not used.

PART 3. EXECUTION

3.1 Concrete work shall conform to ACI 301-72 (as revised), as modified by the supplemental requirements below:

A. Strength

1. The strength of concrete shall be 3,000 psi unless otherwise shown on the drawings.

B. Durability

1. All concrete exposed to weather shall be air entrained.

C. Slump

1. Concrete shall be proportional and produced to have a slump of 3 inches with a 1 inch tolerance.

D. Admixtures

1. Air entrainment, mandatory for concrete exposed to weather, may be used. A water reducing admixture (retarding, normal, or accelerating, depending on placing temperature), may be used if approved by the Spring Hill's representative.

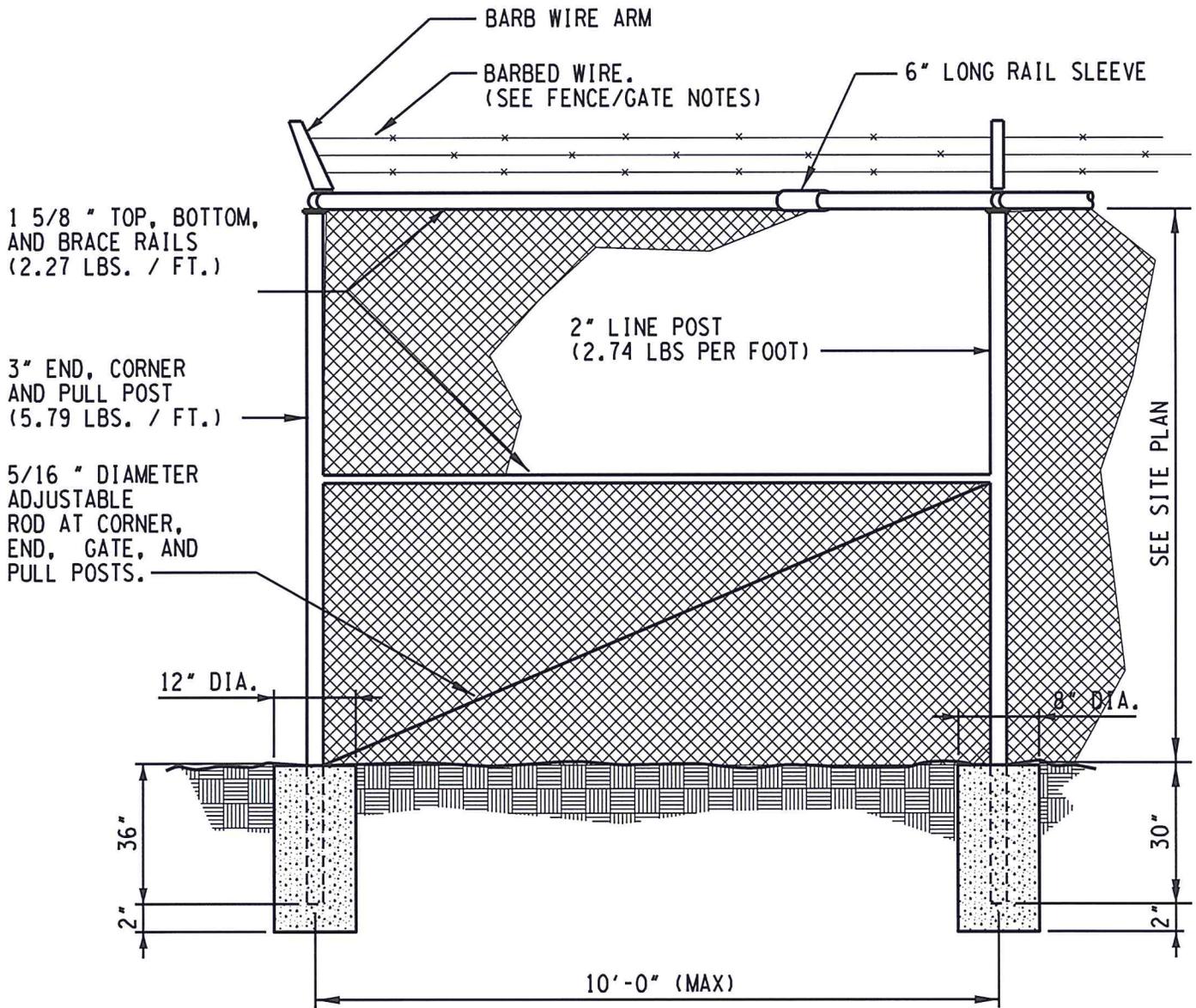
E. Reinforcing Steel

1. Yield strength of reinforcing steel shall be 60,000 psi.

END OF SECTION

**S  
T  
A  
N  
D  
A  
R  
D  
  
D  
R  
A  
W  
I  
N  
G  
S**





TYPICAL FENCE AND GATE DETAIL (1 OF 3)

N.T.S.

85

BARBED WIRE  
(SEE FENCE/GATE NOTES)

WRAP AROUND  
ADJUSTABLE HINGE.

TENSION BARS AND  
HOOK BOLTS BOTH  
SIDES AND AT END  
OF FABRIC.

(SEE SITE PLAN)

3" GATE POST  
(5.79 LBS.  
PER FOOT)

SEE SITE PLAN

5/16" DIAMETER  
ADJUSTABLE ROD

CENTER GATE  
STOP

8" DIA.

2" DIA.

36"

2"

### TYPICAL FENCE AND GATE DETAIL (2 OF 3)

N.T.S.

86

NOTES:

1. PROVIDE POSITIVE TYPE LATCHING DEVICE WITH PROVISIONS FOR PADLOCKING, CENTER PLUNGER ROD, CATCH AND SEMI-AUTOMATIC OUTER CATCHES OR HOLD BACKS.

2. FENCE FABRIC: 6'-0" (72"), 9 GA., 2" MESH, KNUCKLED TOP & BOTTOM, ALUMINIZED WIRE COATED 4 OZ. / S.F. (ASTM A491).

3. FENCE FRAME MEMBERS TO BE TYPE II - LG40 STEEL PIPE (ASTM F1043, GROUP 1C):  
TERMINAL POSTS (CORNER, END, PULL AND GATE POSTS): 3" O.D. (5.79 LBS. / FT.);  
LINE POSTS: 2" O.D. (2.74 LBS. / FT.);  
TOP, BOTTOM AND BRACE RAILS: 1 5/8" O.D. (2.27 LBS. / FT.);  
JOIN WITH 1 5/8" - 6" LONG SLEEVES;  
ALL POSTS SHALL BE HOT DIPPED GALVANIZED WITH MINIMUM 1.8 OZ. / S.F.  
ZINC COATING.

4. SINGLE AND DOUBLE SWING GATE FRAMEWORK: 1 5/8" O.D. (2.27 LBS. / FT.);

5. FITTINGS:

POST CAPS: STEEL, CAST IRON OR ALUMINUM ALLOY;

RAIL ENDS: FORMED STEEL OR IRON;

TIE WIRES: 9 GA., GALVANIZED STEEL OR ALUMINUM; 15" SPACING FOR TERMINAL POSTS AND 24" SPACING FOR TOP AND BRACE RAILS;

HOG RINGS: 9 GA.;

FABRIC AND RAIL BANDS: 12 GA. X 3/4" PRESSED STEEL; 15" SPACING;

STRETCHER BAR: 3/16 " x 3/4 " GALVANIZED STEEL;

LINE POSTS - 9 GA TIE WIRES @ 15" O.C.;

TOP RAILS - 9 GA TIE WIRES @ 24" O.C.

TENSION WIRE-6 GA. CORE WIRE (75,000 PSI TENSILE STRENGTH);

TRUSS RODS AND TIGHTENERS: 5/16" ROD;

FASTENERS: GALVANIZED;

6. BARBED WIRE: PVC COATED PER ASTM F1665 CLASS 2A; TWO STRANDED STEEL WIRE, 13-3/4 GA. TWISTED LINE WIRE; 4 POINT BARBS AT 5" CENTERS AND COATED WITH 0.25 OSF ZINC COATING.

7. BARBED WIRE SUPPORT ARMS: SUITABLE FOR 3 STRANDS OF BARBED WIRE; WITHSTAND A 250 LB DOWNWARD PULL.

8. CONCRETE: 3000 PSI PLANT MIX ONLY. BAG CONCRETE SHALL NOT BE ALLOWED.

9. ALL FENCE FABRIC, POST, RAILS AND HARDWARE SHALL RECEIVE A BLACK PVC COATING OF 6 MIL TO 10 MIL PER ASTM F688 CLASS 2B; PER ASTM F1664 CLASS 2A FOR TENSION WIRE AND ASTM F1665 CLASS 2A FOR BARBED WIRE. FASTENERS SHALL BE PAINTED BLACK.

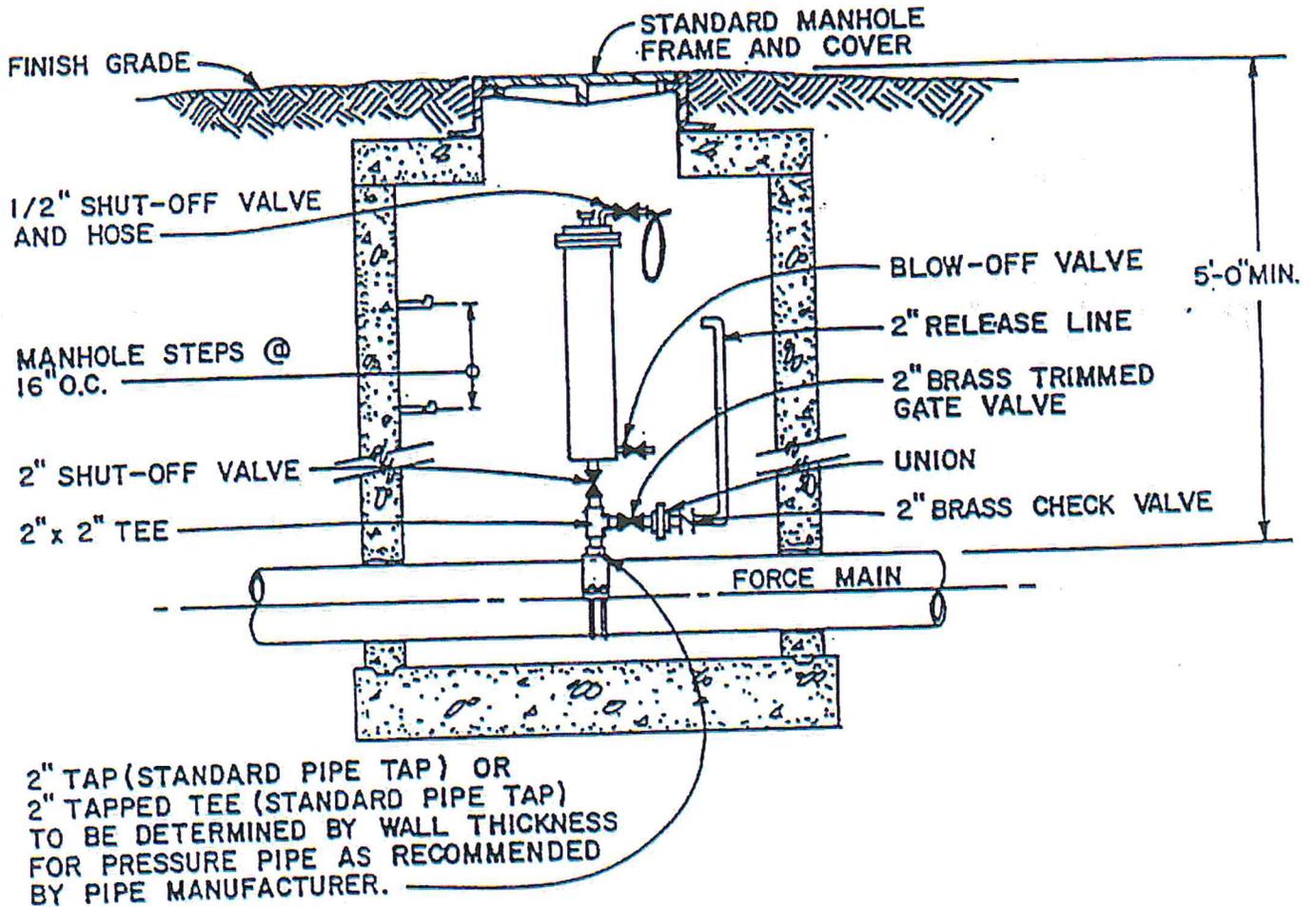
---

TYPICAL FENCE AND GATE DETAIL (3 OF 3)

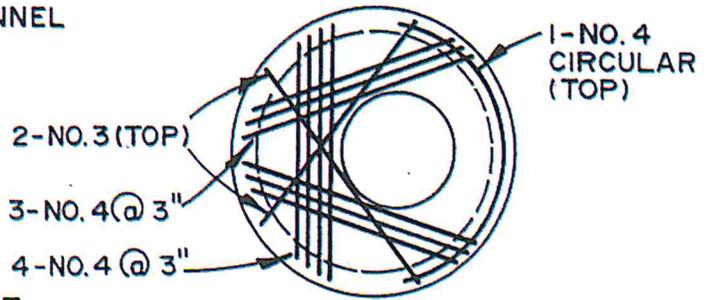
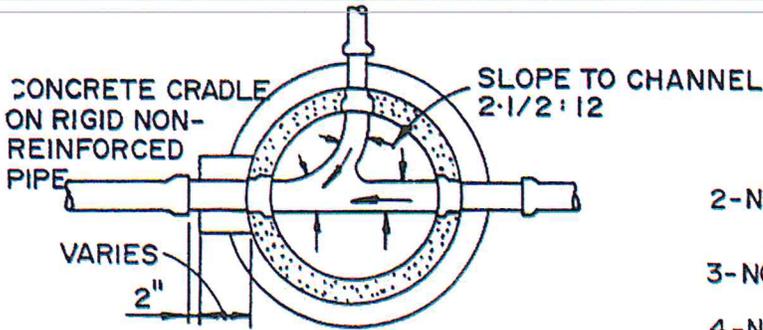
N.T.S.

**NOTES:**

1. AIR RELEASE VALVE TO BE VAL-MATIC "CRISPEN SEWER VALVE", APCO 400 SEWAGE VALVE, OR EQUAL.
2. AIR-VACUUM VALVE INSTALLATION TO BE SIMILAR EXCEPT VALVE TO BE APCO 402 SEWAGE VALVE OR EQUAL.
3. 2" AIR RELEASE LINE, GATE VALVE AND CHECK VALVE ON AIR-VACUUM VALVE ONLY.



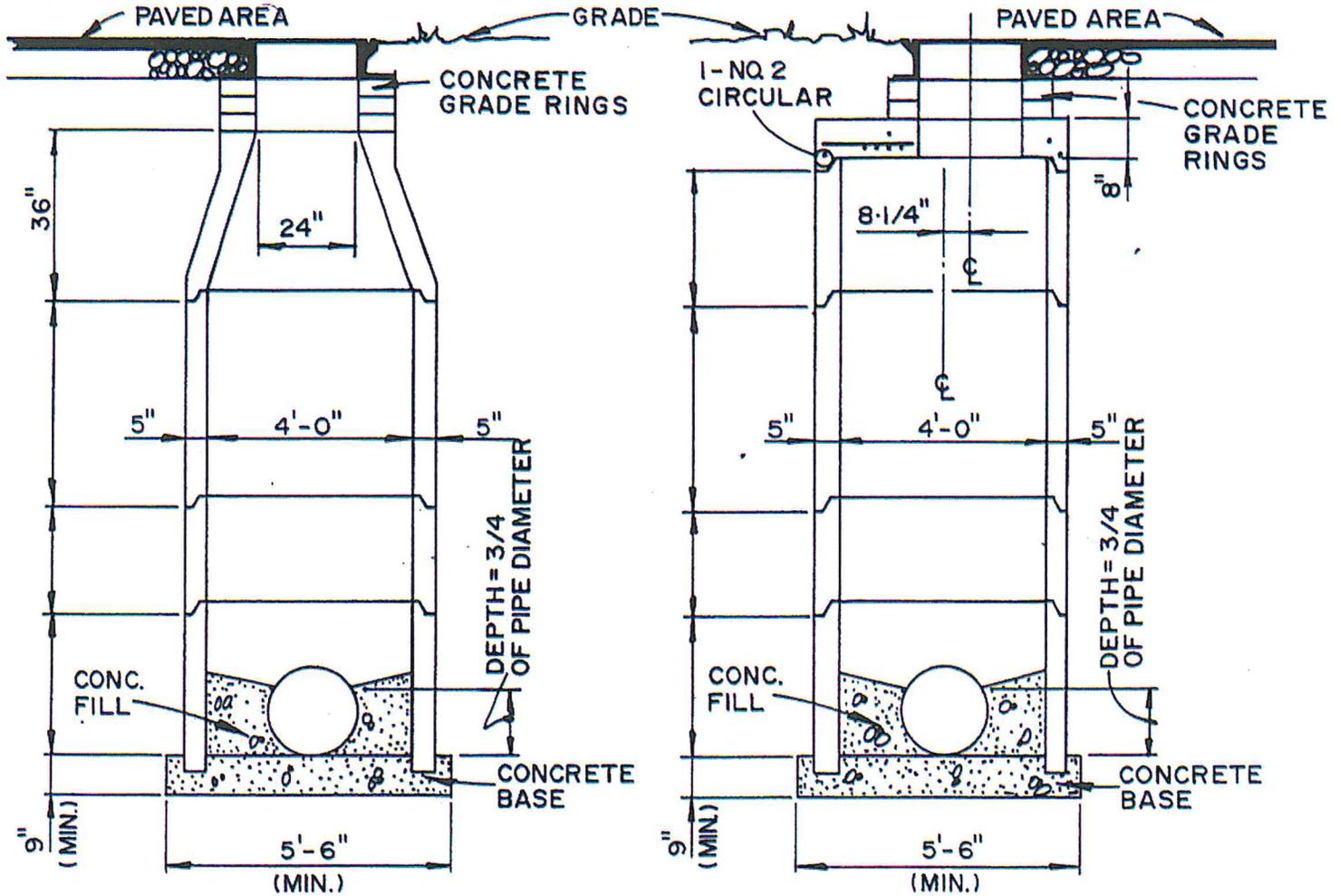
**FORCE MAIN**  
**AIR RELEASE / AIR VACUUM VALVE DETAIL**  
 SCALE: NONE



SLOPE CHANNEL FROM INLET(S) TO OUTLET  
A MINIMUM OF 0.2 FEET ACROSS MANHOLE  
AND NO GREATER THAN 0.5 FEET.

**FORMED INVERT-PLAN**

**PRECAST TOP-PLAN**



**CONCENTRIC CONE**

**FLAT SLAB TOP**

GENERAL NOTES

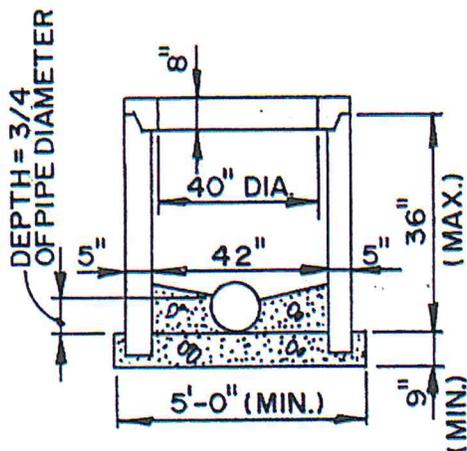
1. ALL MATERIALS, DESIGN, MANUFACTURE, PHYSICAL TEST REQUIREMENTS, FINISH, MARKING, INSPECTION, REJECTION & REPAIRS TO MEET ASTM C478 FOR PRECAST REINFORCED CONCRETE MANHOLE RISERS & TOPS EXCEPT AS MAY BE MODIFIED IN THESE SPECIFICATIONS.
2. MANHOLE STEPS TO BE CAST IN PLACE.
3. KOR-N-SEAL BOOT CONNECTORS OR APPROVED EQUAL REQUIRED.

**STANDARD PRECAST CONCRETE MANHOLES**

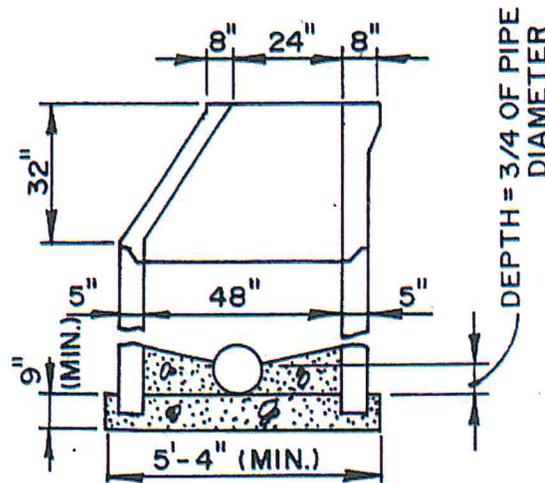
SCALE: NONE

(SHEET 1 OF 2)

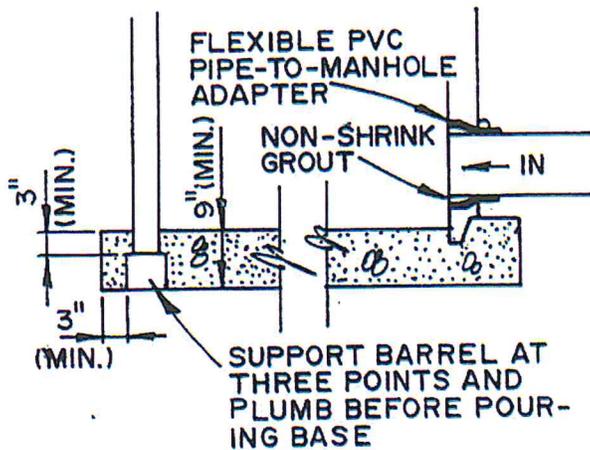
**GS-1**



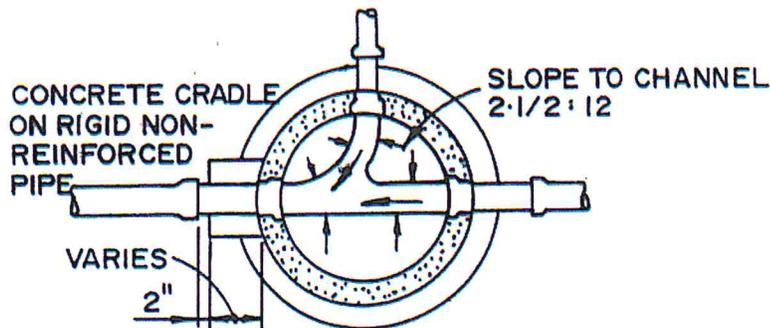
**SPECIAL SHALLOW MANHOLE**



**ECCENTRIC CONE**



**BARREL-TO-BASE DETAIL**



SLOPE CHANNEL FROM INLET(S) TO OUTLET A MINIMUM OF 0.2 FEET ACROSS MANHOLE AND NO GREATER THAN 0.5 FEET.

**FORMED INVERT-PLAN**

**GENERAL NOTES**

1. ALL MATERIALS, DESIGN, MANUFACTURE, PHYSICAL TEST REQUIREMENTS, FINISH, MARKING, INSPECTION, REJECTION & REPAIRS TO MEET ASTM C478 FOR PRECAST REINFORCED CONCRETE MANHOLE RISERS & TOPS EXCEPT AS MAY BE MODIFIED IN THESE SPECIFICATIONS.
2. MANHOLE STEPS TO BE CAST IN PLACE.
3. KOR-N-SEAL BOOT CONNECTORS OR APPROVED EQUAL REQUIRED.

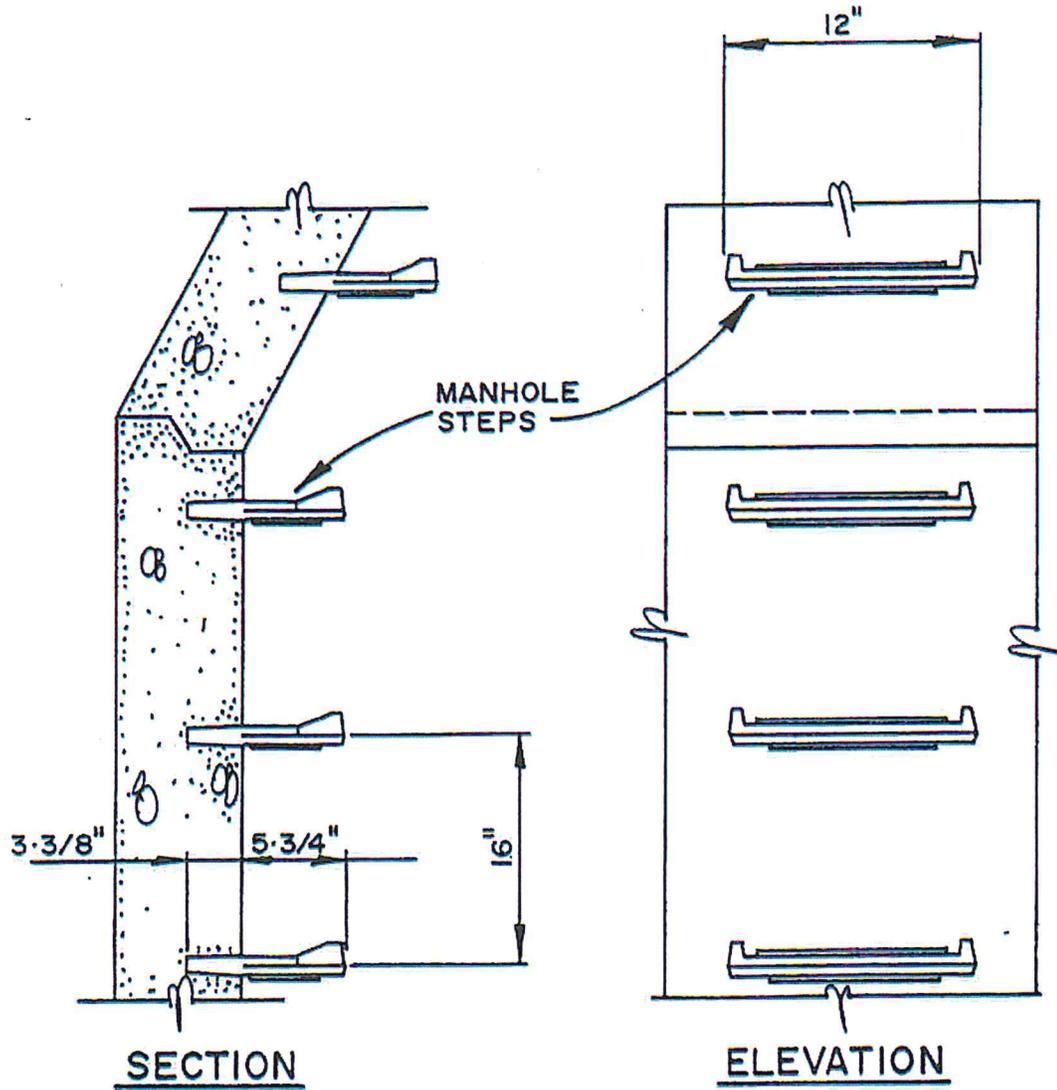
**STANDARD PRECAST CONCRETE MANHOLES**

SCALE: NONE

(SHEET 2 OF 2)

**NOTE:**

MANHOLE STEPS TO BE 3/8" STEEL REINFORCED ROD  
ENCAPSULATED IN POLYPROPYLENE PLASTIC OR EQUAL.

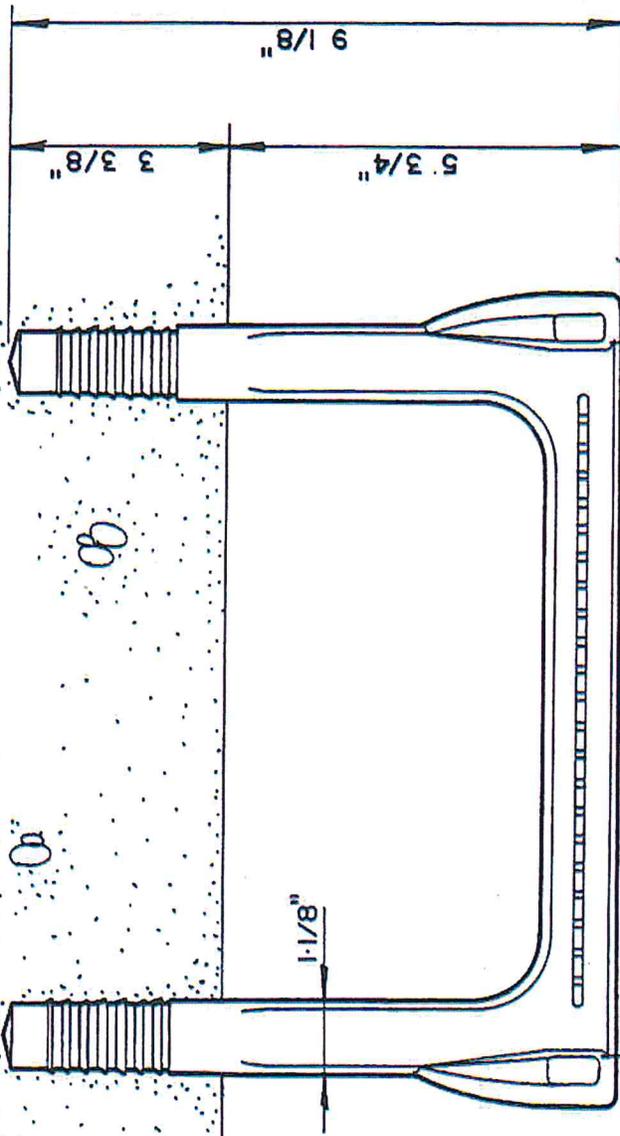


**TYPICAL MANHOLE STEP DETAIL**

SCALE: NONE

NOTE:

FOOT SURFACE TO BE NON-SLIP TEXTURED AND STEEL REINFORCED.

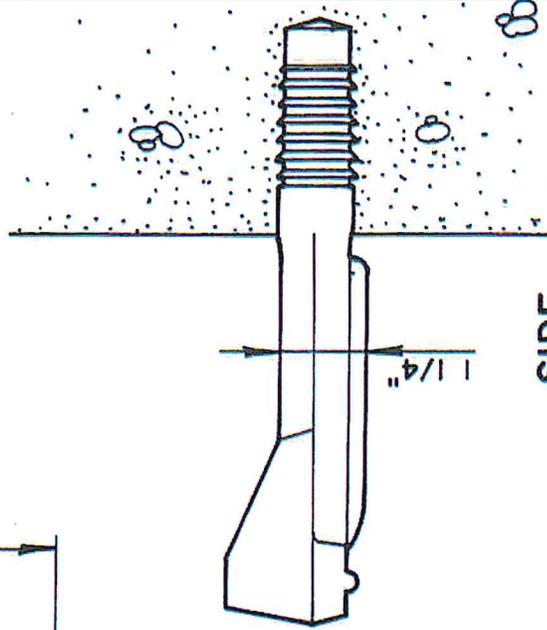


TOP

10 3/4"

12"

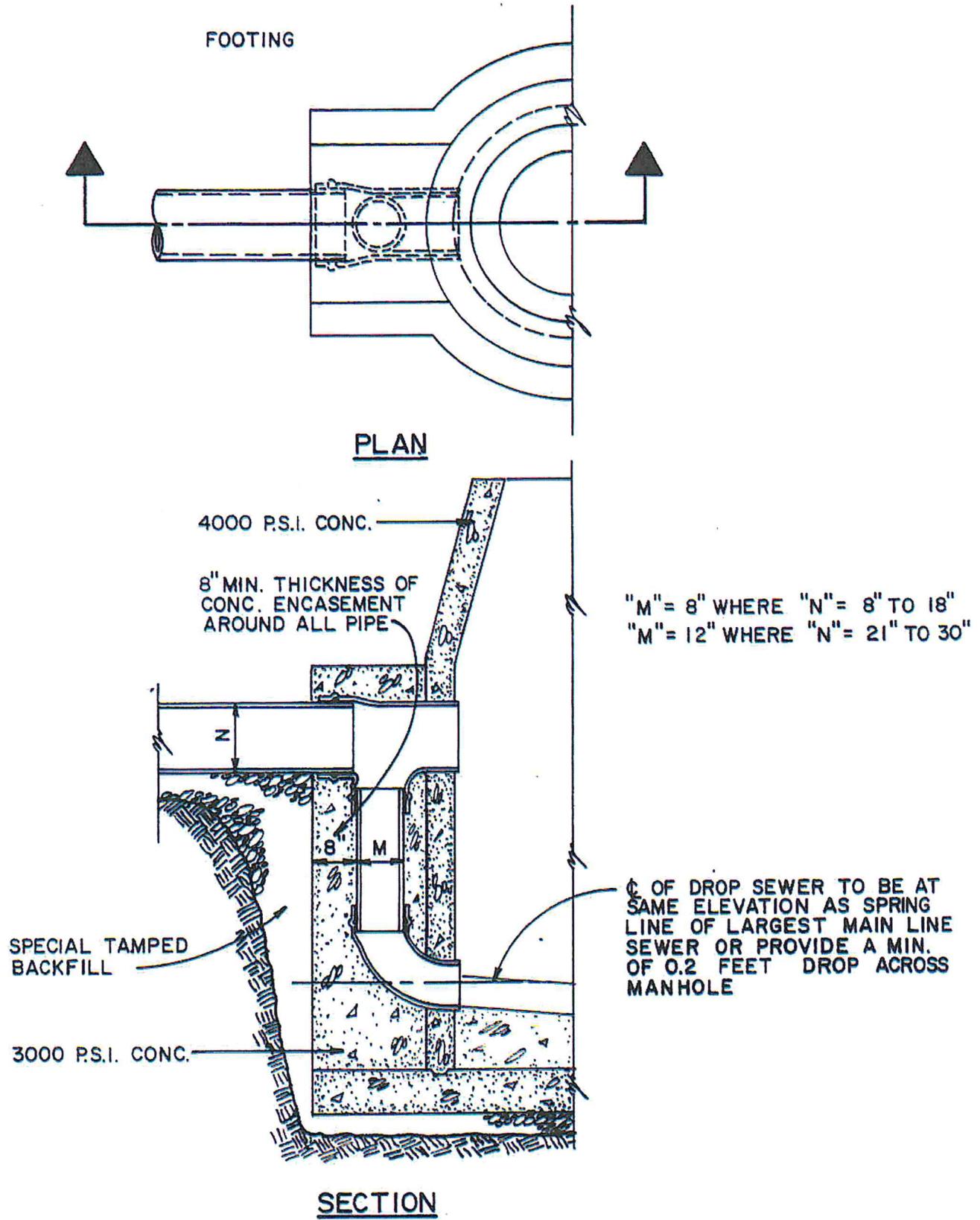
FRONT



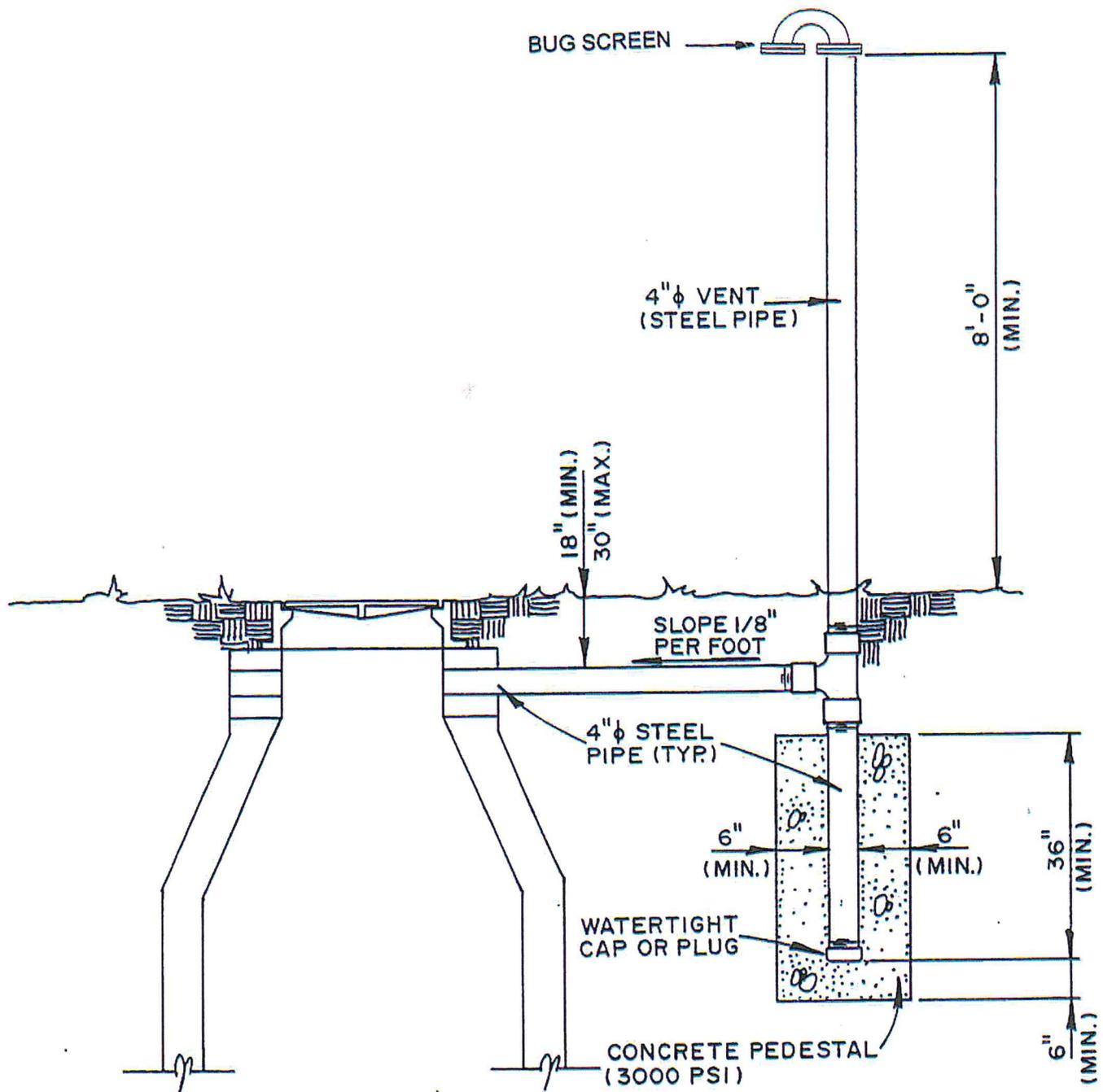
SIDE

MANHOLE STEP DETAIL

SCALE: NONE



**DROP PIPE ASSEMBLY**  
**FOR**  
**STANDARD MANHOLES**  
 SCALE: NONE



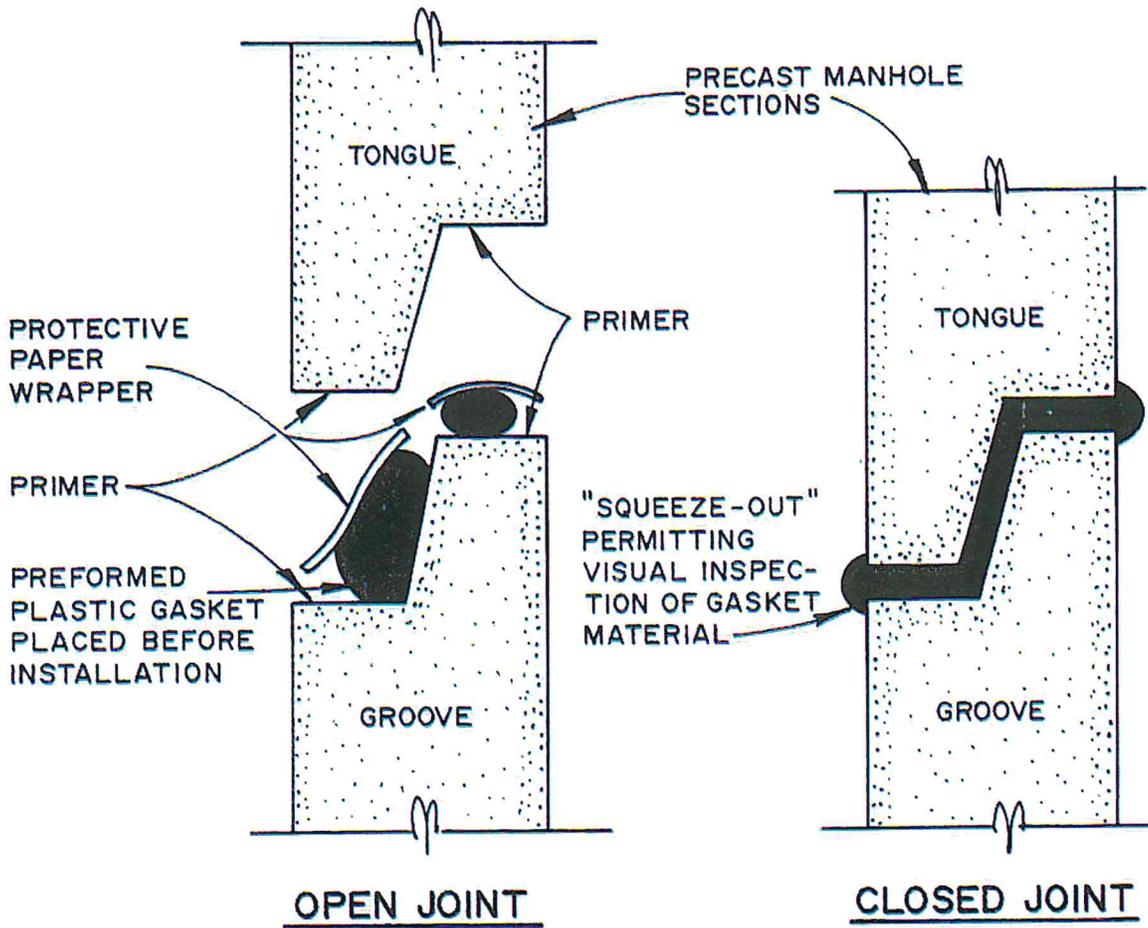
**GENERAL NOTES:**

1. VENT TO BE LOCATED BEHIND CURB OR WALK AND FAR ENOUGH FROM ROADWAY SO AS TO PREVENT TRAFFIC FLOW OBSTRUCTION. OR AS NOTED ON THE PLANS.
2. ALL VENT PIPING IS TO BE PRIMED WITH ONE COAT OF RED PRIMER AND TWO COATS OF DARK GREEN ENAMEL.
3. TOP OF VENT TO BE A MINIMUM OF 8'-0" ABOVE GRADE OR AS NOTED ON THE PLANS.

**STANDARD MANHOLE VENT**  
SCALE: NONE

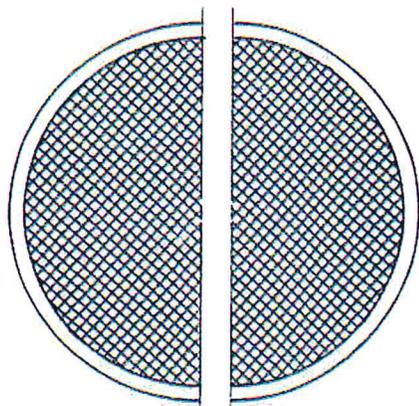
**NOTE:**

WHERE GASKET MATERIAL DOES NOT PROTRUDE FROM JOINTS, POINT UP JOINT WITH GROUT, WHETHER INSIDE OR OUTSIDE.

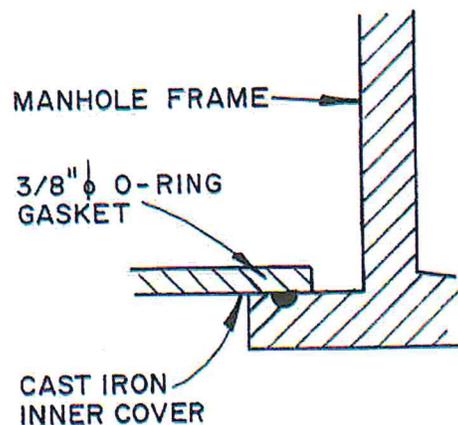


**PLASTIC GASKET JOINT DETAIL**  
**FOR PRECAST MANHOLE SECTIONS**

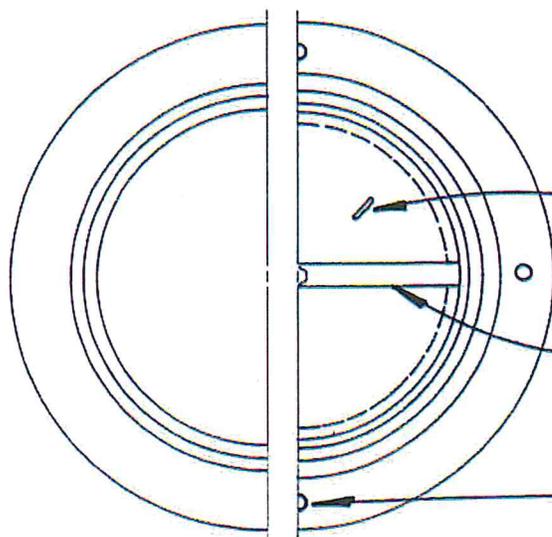
SCALE: NONE



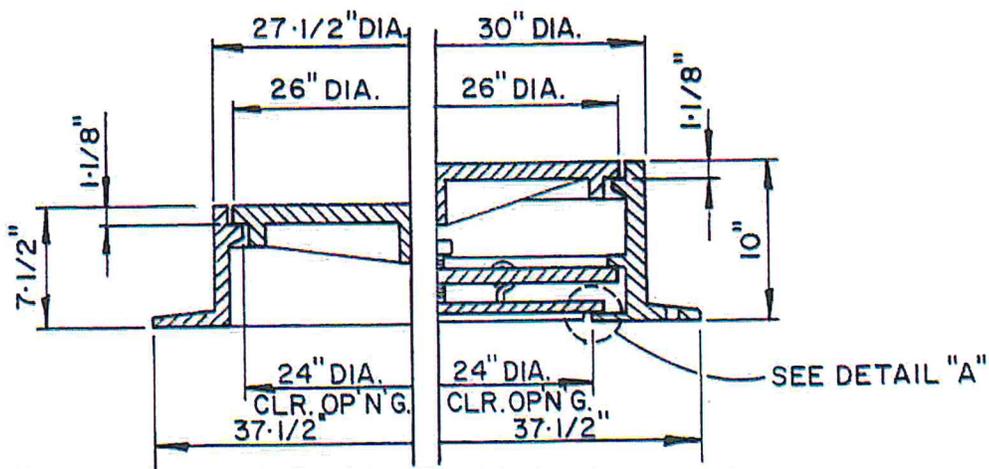
**COVER**



**DETAIL "A"**  
SCALE: NONE



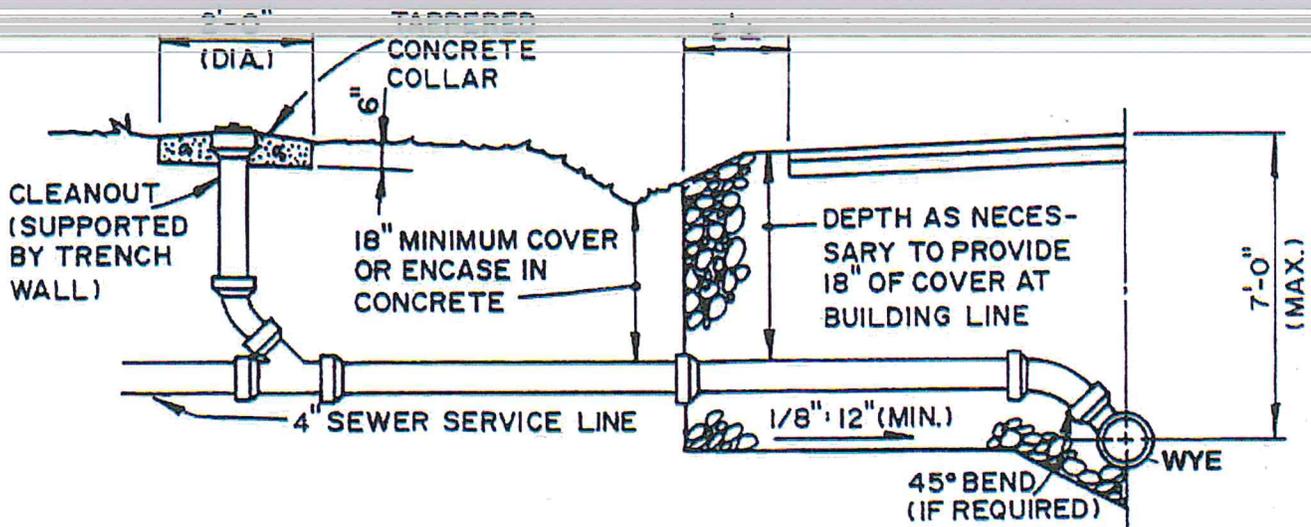
**FRAME**



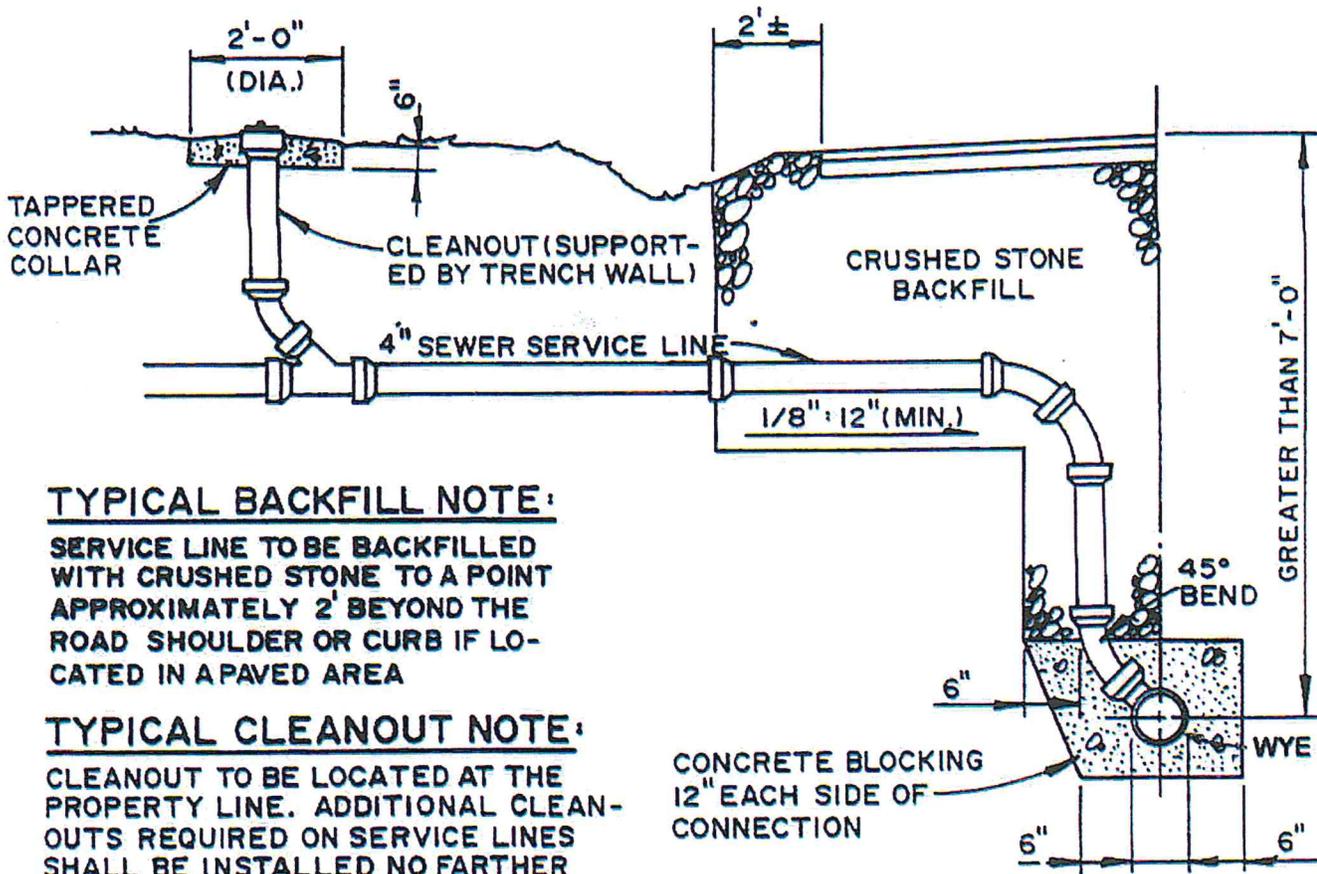
**STANDARD      WATERTIGHT**

**FRAME & COVER**

SCALE: NONE



**TEE BRANCH (SEWER DEPTH TO 7'-0" MAX.)**



**TYPICAL BACKFILL NOTE:**

SERVICE LINE TO BE BACKFILLED WITH CRUSHED STONE TO A POINT APPROXIMATELY 2' BEYOND THE ROAD SHOULDER OR CURB IF LOCATED IN A PAVED AREA

**TYPICAL CLEANOUT NOTE:**

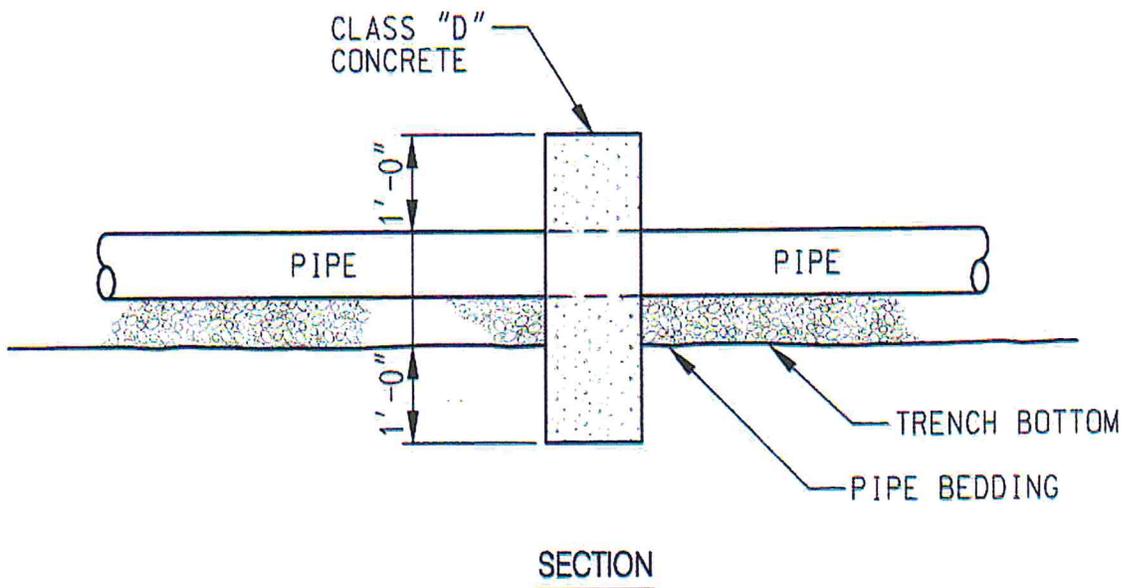
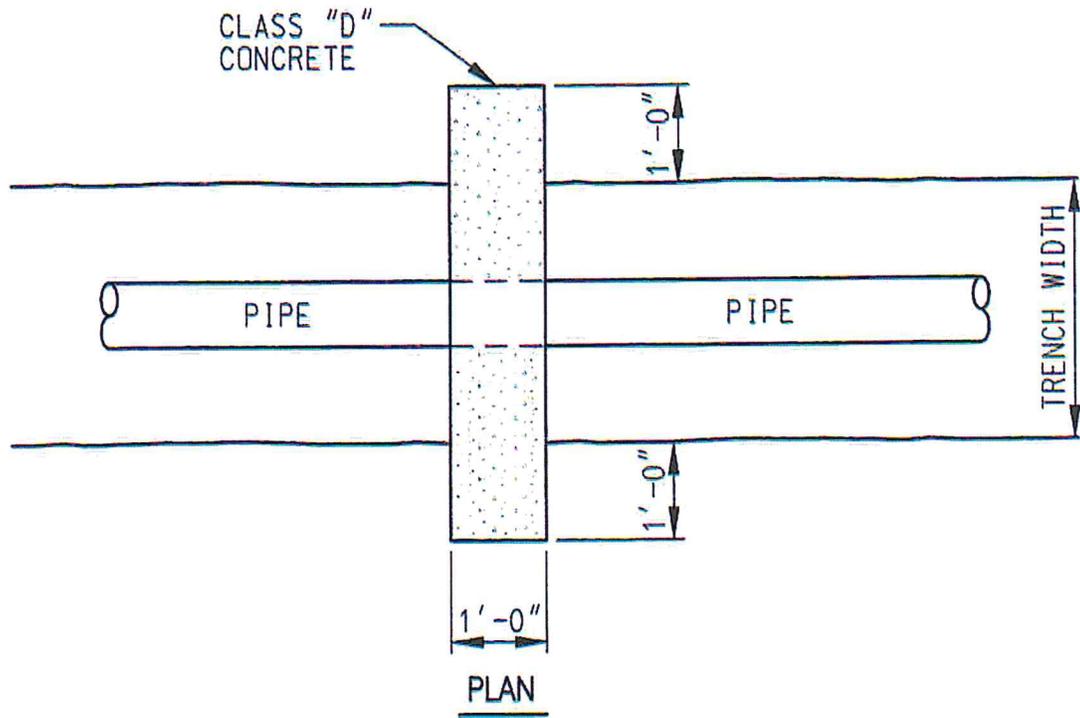
CLEANOUT TO BE LOCATED AT THE PROPERTY LINE. ADDITIONAL CLEANOUTS REQUIRED ON SERVICE LINES SHALL BE INSTALLED NO FARTHER THAN 75 FEET APART.

**TEE BRANCH (SEWER DEPTH GREATER THAN 7'-0")**

**SANITARY SEWER SERVICE LINE CONNECTION DETAILS**

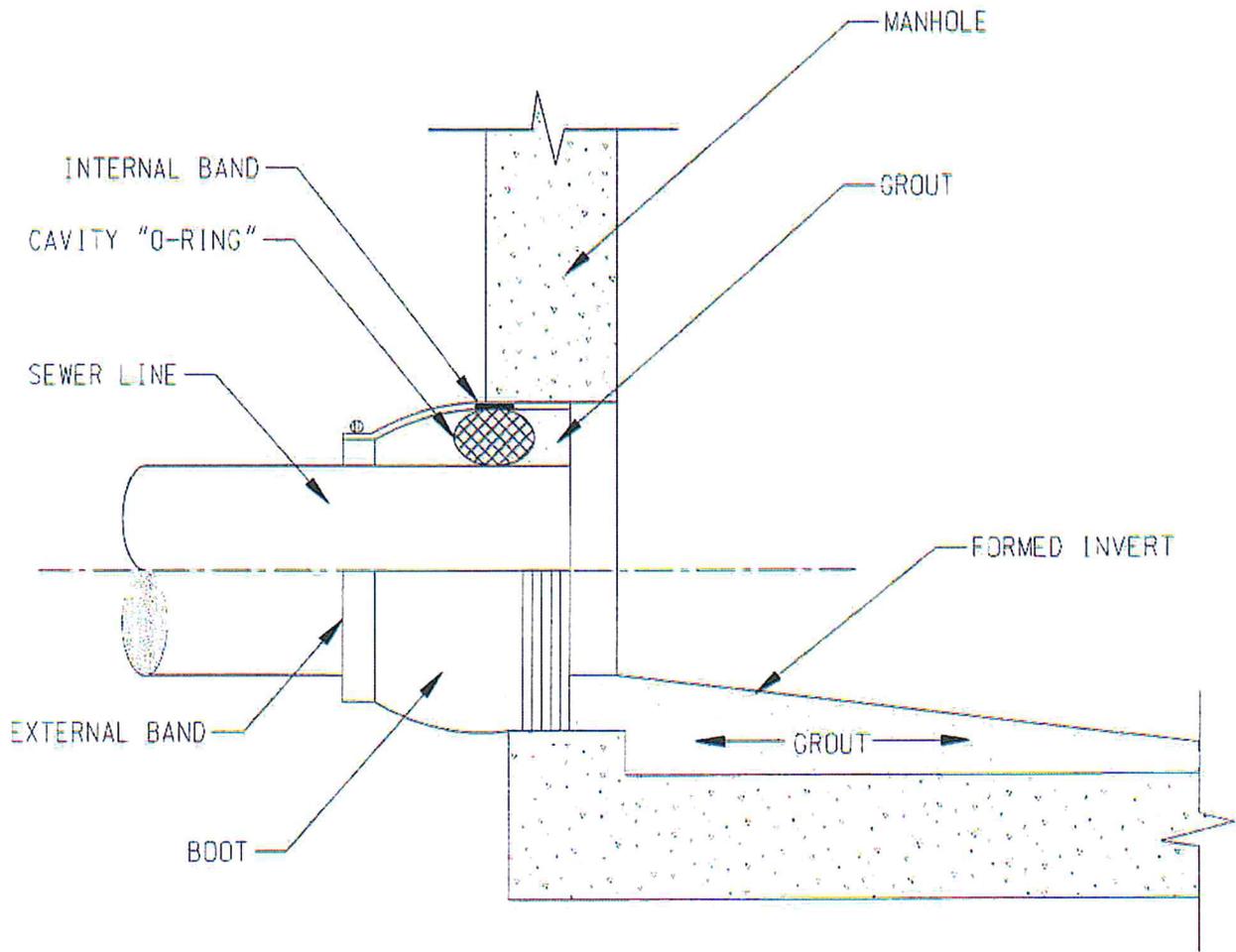
(TYPICAL FOR 10" MAINS AND SMALLER)

SCALE: NONE



**CONCRETE WATERSTOP FOR GRAVITY SEWER LINES DETAIL**

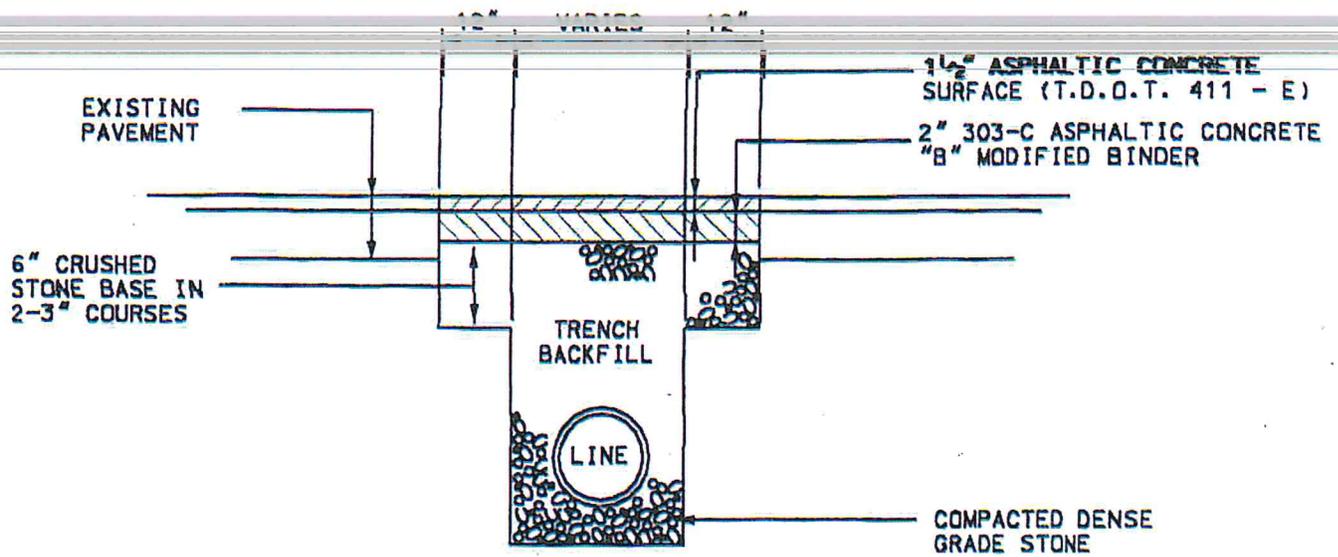
N.T.S.



**PIPE TO MANHOLE CONNECTION DETAIL**

N.T.S.

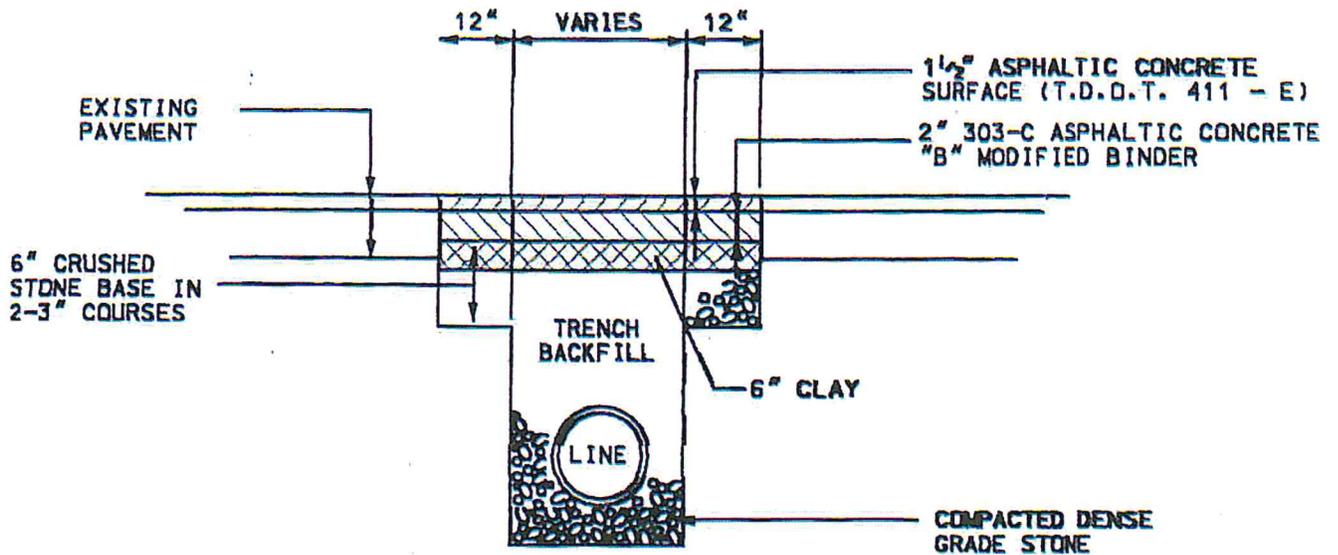
8



INSTALLATION PERPENDICULAR TO STREET

NOTE:

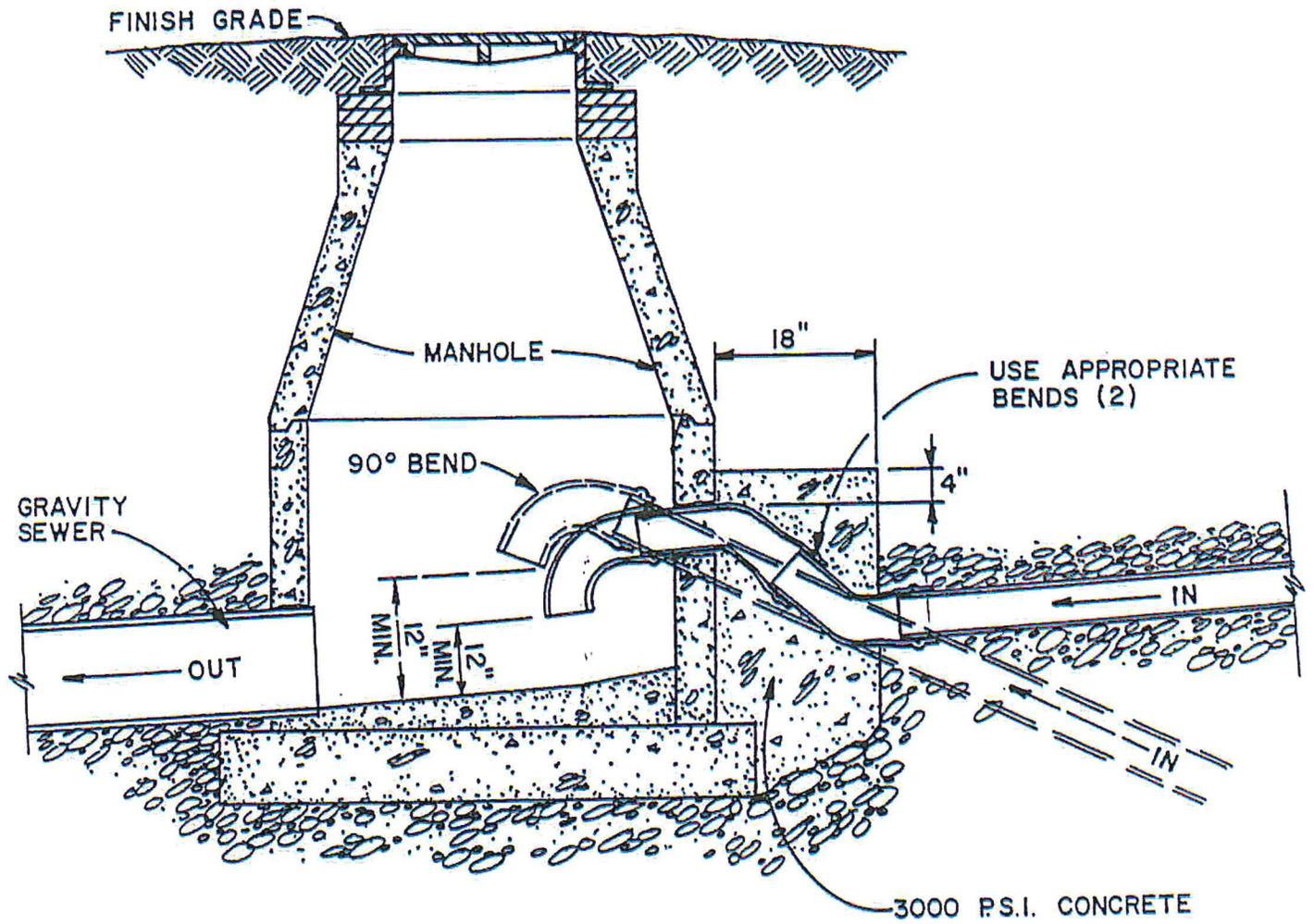
EDGE OF PAVEMENT TO BE TRIMMED A MINIMUM OF 12" BEYOND EACH SIDE OF TRENCH WIDTH TO OBTAIN NEAT LINES. COLD MIX TO BE PLACED AS A TEMPORARY SURFACE WITHIN 48 HOURS OF MAKING ROAD CROSSING.



PAVEMENT REPLACEMENT DETAIL  
(BITUMINOUS BASE WITH SURFACE)

NO SCALE

PR-1A



**STANDARD CONNECTION OF FORCE  
MAIN TO MANHOLE**

SCALE : NONE

101